

ABBREVIATIONS

(A)	ABOVE	(N)	NEW
AB	ANCHOR BOLT	oc	ON CENTER
AB'S	ANCHOR BOLTS	opp	OPPOSITE HAND
ADD'L	ADDITIONAL	OSSC	OREGON STRUCTURAL SPECIALTY CODE
(B)	BELOW		
BF	BRACED FRAME	OWJ	OPEN WEB JOIST
BOD	BOTTOM OF DECK	PREP	PREPARE, PREPARATION
CONC.	CONCRETE	PLF	POUNDS PER LINEAR FOOT
CONN.	CONNECTION		
d	DEPTH	PSL	PARALLAM PSL BY TRUS-JOIST MACMILLAN
DEMO'D	DEMOLISHED		
DIA., DIAM	DIAMETER	REINF	REINFORCEMENT
(E)	EXISTING	SIM	SIMILAR
EA	EACH	SOG	SLAB ON GRADE
ELEV	ELEVATION	STD	STANDARD
EOS	EDGE OF SLAB	STRUCT	STRUCTURE, STRUCTURAL
EXT	EXTERIOR	T&B	TOP AND BOTTOM
FF	FINISHED FLOOR	TOF	TOP OF FOOTING
FG	FINISHED GRADE	TOS	TOP OF STEEL
FLR	FLOOR	TOW	TOP OF WALL
FTNG	FOOTING	TYP.	TYPICAL
HOR	HORIZONTAL	UBC	UNIFORM BUILDING CODE
IBC	INTERNATIONAL BUILDING CODE	UNO	UNLESS NOTED OTHERWISE
INFO	INFORMATION		
INT	INTERIOR	(V)	VERIFY, TO BE VERIFIED
LEV	LEVEL		
LSL	TIMBERSTRAND LSL BY TRUS-JOIST MACMILLAN	VERT	VERTICAL
LVL	MICROLLAM LVL BY TRUS-JOIST MACMILLAN	w	WIDE, WIDTH
		w/	WITH
MF	MOMENT FRAME	WWF	WELDED WIRE FABRIC
(N)	NEW	@	AT

GENERAL NOTES

CODE REQUIREMENTS: CONFORM TO THE 2014 OREGON STRUCTURAL SPECIALTY CODE.

STRUCTURAL SUMMARY: PREMANUFACTURED METAL BUILDING ON CONVENTIONAL SHALLOW FOUNDATION. METAL BUILDING DESIGN SHALL CONFORM WITH THE REQUIREMENTS OF THESE DRAWINGS.

DESIGN FORCES ARE BASED UPON REQUIREMENTS OF THE 2014 OREGON STRUCTURAL SPECIALTY CODE WITH THE DESIGN LOADS LISTED BELOW IN ADDITION TO DEAD LOADS:

SNOW LOAD:	Pg = 10 PSF, Pf = 25 PSF	
ROOF LIVE LOAD:	20 POUNDS PER SQUARE FOOT	
FOUNDATION DESIGN:	BASED ON THE REQUIREMENTS DESCRIBED IN "REVISED GEOTECHNICAL REPORT: URBAN FORESTRY YARD IMPROVEMENTS, PORTLAND OREGON, PP&R PROJECT P30076 DATED DECEMBER 10, 2018" BY CITY OF PORTLAND ENVIRONMENTAL SERVICES MATERIAL TESTING LABORATORY. ALLOWABLE BEARING PRESSURE = 1000 PSF	
	SITE CLASS = E	
WIND:	135 MPH EXP. C, Iw = 1.0, GCPI = 0.55	
SEISMIC:	SDS = 0.608, SEISMIC USE CATEGORY D, R=3.5, I = 1.0	

TEMPORARY CONDITIONS: THE STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE. THE DRAWINGS DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE STRUCTURE IS DESIGNED TO BE STABLE AS COMPLETED. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, ERECTION, AND INSPECTION OF TEMPORARY SHORES, BRACES, ETC. THAT SUPPORT THE STRUCTURE AGAINST ALL ANTICIPATED LOADS INCLUDING GRAVITY, WIND, AND LATERAL EARTH PRESSURES UNTIL THE COMPLETION OF THE STRUCTURE. TEMPORARY CONSTRUCTION LIVE LOADS SHALL NOT EXCEED THOSE IN THE DESIGN CRITERIA.

VERIFICATION OF SITE CONDITIONS AND EXISTING CONDITIONS: THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS AND INFORM THE ARCHITECT AND ENGINEER OF ANY DISCREPANCIES BETWEEN FIELD CONDITIONS AND INFORMATION SHOWN IN THE DRAWINGS.

SUBMITTAL REQUIREMENTS: THE CONTRACTOR SHALL SUBMIT TO THE ARCHITECT LAYOUT DRAWINGS AND SUPPORTING CALCULATIONS FOR THE PRE-MANUFACTURED BUILDING. ENGINEERING CALCULATIONS SHALL BE SEALED BY AN ENGINEER REGISTERED IN THE STATE OF OREGON. THE BUILDING MANUFACTURER SHALL BE ACCREDITED BY THE METAL BUILDING MANUFACTURERS ASSOCIATION (MBMA) WITH A QUALITY ASSURANCE PROGRAM VERIFIED BY A THIRD-PARTY INSPECTION AGENCY.

FIELD ENGINEERED DETAILS THAT DIFFER FROM THOSE DRAWN SHALL BE DESIGNED AND STAMPED BY AN ENGINEER REGISTERED IN THE STATE OF OREGON AND SUBMITTED TO THE ENGINEER OF RECORD AND THE JURISDICTION FOR REVIEW AND APPROVAL.

SPECIAL INSPECTION: THE OWNER SHALL RETAIN AN ICC CERTIFIED SPECIAL INSPECTOR TO PROVIDE INSPECTION OF THE FOLLOWING ITEMS PER ICC SECTION 1701:

ITEM	INSPECTION FREQUENCY	NOTES
SOILS: GRADING, SHORING, EXCAVATION, STRUCTURAL FILL & FINAL FOUNDATION PREP	PERIODIC	BY GEOTECHNICAL ENGINEER
CONCRETE: REINFORCEMENT EMBEDDED ANCHORS TEST SPECIMENS PLACEMENT/GROUTING	PERIODIC PERIODIC CONTINUOUS CONTINUOUS	BUILDING FOOTINGS AND STEM WALL DOWELS BUILDING FOOTINGS ONLY BUILDING FOOTINGS ONLY
MASONRY: VERIFICATION OF SLUMP FLOW AND VSI AS DELIVERED TO THE SITE FOR SELF-CONSOLIDATING GROUT	PERIODIC	
VERIFICATION OF SITE-PROPORTIONED MORTAR	CONTINUOUS	
CONSTRUCTION OF MORTAR JOINTS	CONTINUOUS	

STRUCTURAL STEEL:

STEEL SHAPE:	GRADE AND YIELD STRENGTH:
WF NOT PART OF LFRS	ASTM A572 GRADE 50, ASTM A992, GRADE 50
CHANNELS, PLATES, AND ANGLES	ASTM A36
HSS	ASTM A500 GRADE B, Fy = 46 KSI

DESIGN, FABRICATION, AND ERECTION SHALL FOLLOW THE "AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" WITH THE "COMMENTARY" AND "CODE OF STANDARD PRACTICE".

BOLTS SHALL BE ASTM A325 HIGH STRENGTH BOLTS UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE FULLY TIGHTENED.

WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY (AWS) CODES FOR ARC AND GAS WELDING. ALL WELDING SHALL BE PRE-QUALIFIED AND PERFORMED WITH A WELDING PROCEDURE SPECIFICATION (WPS) PER AWS D1.1. WELDS SHALL BE MADE USING E70XX ELECTRODES AND SHALL BE 3/16" MINIMUM UNLESS OTHERWISE NOTED. WELDING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS.

SUBGRADE PREPARATION REQUIREMENTS:

PREPARE THE SITE AND PREPARE SUBGRADE FOR FOOTINGS AND BUILDING SLABS AS RECOMMENDED IN THE GEOTECHNICAL REPORT. FILLS SHOULD ONLY BE PLACED OVER SUBGRADE THAT HAS BEEN PREPARED IN CONFORMANCE WITH THE "SITE PREPARATION" SECTION OF THE GEOTECHNICAL REPORT. STRUCTURAL FILL SHOULD MEET THE SPECIFICATIONS PROVIDED IN THE OSSC 00330 (EARTHWORK), OSSC 00400 (DRAINAGE AND SEWERS), AND OSSC 02600 (AGGREGATES), DEPENDING ON THE APPLICATION.

CONCRETE:

CONCRETE WORK SHALL COMPLY WITH CHAPTER 19 OF THE IBC. CONTRACTOR SHALL SUBMIT TEST DATA. MINIMUM COMPRESSIVE STRENGTH SHALL BE:

f'c = 3000 PSI FOR FOOTINGS AND WALLS
f'c = 4000 PSI FOR SLABS ON GRADE/ALL CONCRETE.
WATER CEMENT RATIO BY WEIGHT NOT TO EXCEED 0.5. MAXIMUM AGGREGATE SIZE = 3/4". MAXIMUM SLUMP = 4". MINIMUM CEMENT PER CUBIC YARD = 400 LBS.

AIR ENTRAIN 5% FOR CONCRETE EXPOSED TO WEATHER.

EXISTING CONCRETE SURFACES THAT ARE BONDED TO NEW CONCRETE SHALL BE CLEANED AND ROUGHENED TO 1/4" AMPLITUDE.

REINFORCING STEEL: REINFORCING STEEL SHALL BE GRADE Fy = 60 KSI IN CONFORMANCE WITH ASTM A615, INCLUDING S1. LAP ALL REINFORCING BARS A MINIMUM OF 24" OR AS NOTED. REINFORCING STEEL SHALL HAVE THE FOLLOWING CLEARANCES:

FOOTINGS	3" CLR
SLAB ON GRADE	1" CLR
RETAINING WALLS	1 1/2" CLR

LAP SPLICE SCHEDULE: REFER TO IBC CHAPTER 19 FOR REQUIREMENTS OF CLASS "A" AND CLASS "B" LAP SPLICES. FOR OTHER CONDITIONS REFER TO TABLE BELOW FOR MINIMUM LAP LENGTHS:

#4	25" LAP
#5	31" LAP
#6	37" LAP
#8	48" LAP

CONCRETE ANCHORS: CAST-IN-PLACE ANCHORS SHALL CONFORM WITH ASTM F1554, GRADE 36. EXPANSION ANCHORS SHALL BE SIMPSON STRONG-BOLT OR AN APPROVED SUBSTITUTE WITH ICC EVALUATION REPORT. DO NOT CUT REINFORCEMENT FOR EXPANSION ANCHOR PLACEMENT. EMBEDMENTS EXPOSED TO WEATHER SHALL BE GALVANIZED. EPOXY ADHESIVE SHALL BE SIMPSON SET XP OR AN APPROVED SUBSTITUTE WITH ICC EVALUATION REPORT.

MATERIALS EMBEDDED IN CONCRETE: CONDUITS, PIPES, SLEEVES, AND FITTINGS OF ANY MATERIAL NOT HARMFUL TO CONCRETE AND WITHIN LIMITATION OF ACI SECTION 6.3 SHALL BE PERMITTED TO BE EMBEDDED IN CONCRETE WITH APPROVAL OF ENGINEER. CONTRACTOR TO SUBMIT FOR APPROVAL DRAWINGS OF ALL PROPOSED EMBEDDED CONDUITS, PIPES, SLEEVES, AND FITTINGS PRIOR TO PLACING CONCRETE. SLEEVES, PIPES OR CONDUITS OF ALUMINUM SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE UNLESS EFFECTIVELY COATED. CONDUITS AND PIPES EMBEDDED IN CONCRETE SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN 1/3 THE OVERALL THICKNESS OF SLAB. THEY SHALL NOT BE SPACED CLOSER THAN 3 OUTSIDE DIAMETERS OR WIDTHS ON CENTER. THEY SHALL NOT IMPAIR SIGNIFICANTLY THE STRENGTH OF THE CONSTRUCTION. CONCRETE COVER FOR PIPES, CONDUITS, AND FITTINGS SHALL NOT BE LESS THAN 1-1/2 INCHES FOR CONCRETE EXPOSED TO EARTH OR WEATHER, NOR 3 INCHES FOR CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND.

MASONRY:

- CONCRETE MASONRY UNITS AND ASSEMBLIES SHALL BE FULLY GROUTED WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,500 PSI AT 28 DAYS.
- MORTAR SHALL BE TYPE S WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI.
- GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS.
- LAP ALL BARS 36" MINIMUM, OR 48" BAR DIAMETERS, WHICHEVER IS GREATER.
- ALL GROUTING TO BE LOW LIFT. CONTRACTOR VERIFY ASSEMBLY BY PRISM TESTS.

SAWN LUMBER: SAWN LUMBER SHALL CONFORM TO THE WESTERN WOOD PRODUCTS ASSOCIATION GRADING RULES. LUMBER SHALL BE KD (KILN-DRIED) WITH MAXIMUM MOISTURE CONTENT OF 15% TO BE SUBMITTED BY CONTRACTOR. PROTECT STORED WOOD ON SITE FROM MOISTURE. THE SPECIES AND GRADE SHALL BE AS NOTED BELOW:

DIMENSIONAL LUMBER 2" TO 4" THICK	DOUGLAS FIR-LARCH #2
HEADERS/BEAMS	DOUGLAS FIR-LARCH #1
POSTS	DOUGLAS FIR-LARCH #1

NAILING NOT SHOWN SHALL BE AS INDICATED ON FASTENING SCHEDULE. ALL BOLTS AND LAG SCREWS (HAND TIGHTEN) SHALL BE INSTALLED WITH STANDARD CUT WASHERS. CUTTING AND NOTCHING OF STUDS SHALL CONFORM TO IBC CHAPTER 23. ALL LUMBER IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED. FRAMING ACCESSORIES AND STRUCTURAL FASTENERS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE.


FASTENING SCHEDULE MINIMUM U.N.O (DETAILS SUPERSEDE) CONFORMS W/ OSSC 2014 TABLE 2304.9.1. REFER TO IBC FOR CONDITIONS NOT SHOWN.		
CONNECTION	FASTENING	LOCATION
1. JOIST TO SILL OR GIRDER.	3-8d COMMON (2 1/2"x0.131") 3-3"x0.131" NAILS 3-3" 14 GAGE STAPLES	TOENAIL.
2. BRIDGING TO JOIST.	2-8d COMMON (2 1/2"x0.131") 2-3"x0.131" NAILS 2-3" 14GAGE STAPLES	TOENAIL EA. END.
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST.	2-8d COMMON (2 1/2"x0.131")	FACE NAIL.
4. WIDER THAN 1"x6" SUBFLOOR TO EA JOIST.	3-8d COMMON (2 1/2"x0.131")	FACE NAIL.
5. 2" SUBFLOOR TO JOIST OR GIRDER.	2-16d COMMON (3 1/2"x0.162")	BLIND AND FACE NAIL.
6. SOLE PLATE TO JOIST OR BLOCKING. SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL.	16d (3 1/2"x0.135") AT 16" oc. 3"x0.131" NAILS AT 8" oc. 3" 14GAGE STAPLES @ 12" oc. 3-16d (3 1/2"x0.135") AT 16" 4-3"x0.131" NAILS AT 16" oc. 4-3" 14GAGE STAPLES PER 16"	TYPICAL FACE NAIL. BRACED WALL PANELS.
7. TOP PLATE TO STUD.	2-16d COMMON (3 1/2"x0.162) 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	TOENAIL EA. END.
8. STUD TO SOLE PLATE.	4-8d COMMON (2 1/2"x0.131") 4-3"x0.131" NAILS 3-3" 14GAGE STAPLES	TOENAIL.
DOUBLE TOP PLATES.	2-16d COMMON (3 1/2"x0.162") 3-3" 0.131" NAILS 3-3" 14GAGE STAPLES	END NAIL.
	16d (3 1/2"x0.135") AT 24" oc. 3"x0.131" NAILS @ 8" oc. 3" 14GAGE STAPLES @ 8" oc.	FACE NAIL.
10. DOUBLE TOP PLATES.	16d (3 1/2"x0.135") @ 16" oc 3"x0.131" NAIL AT 12" oc. 3" 14GAGE STAPLES AT 12" oc.	TYP. FACE NAIL.
DOUBLE TOP PLATES.	8-16d COMMON (3 1/2"x0.162") 12-3" 0.131" NAILS 12-3" 14GAGE STAPLES	LAP SPLICE.
11. BLOCKING BETWEEN JOIST OR RAFTERS TO TOP PLATE.	3-8d COMMON (2 1/2"x0.131") 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	TOENAIL.
12. RIM JOIST TO TOP PLATE.	8d (2 1/2"x0.131") AT 6" oc. 3"x0.131" NAIL AT 6" oc. 3" 14 GAGE STAPLE AT 6" oc.	TOENAIL.
13. TOP PLATES, LAPS AND INTERSECTIONS.	2-16d COMMON (3 1/2"x0.162") 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	FACE NAIL.
14. CONTINUOUS HEADER, TWO PIECES.	16d COMMON (3 1/2"x0.162")	16" oc. ALONG EDGE.
15. CEILING JOISTS TO PLATE.	3-8d COMMON (2 1/2"x0.131") 5-3"x0.131" NAILS 5-3" 14GAGE STAPLES	TOENAIL.
16. CONTINUOUS HEADER TO STUD.	4-8d COMMON (2 1/2"x0.131")	TOENAIL.
17. CEILING JOISTS, LAPS OVER PARTITIONS (SEE SECT. 2308.10.4.1)	3-16d (3 1/2"x0.162") COMMON MIN., TABLE 2308.10.4.1 4-3"x0.131" NAILS 4-3" 14GAGE STAPLES	FACE NAIL.
18. CEILING JOISTS TO PARALLEL RAFTERS (SEE SECT. 2308.10.1, TABLE 2308.10.1).	3-16d (3 1/2"x0.162") COMMON MIN., TABLE 2308.10.4.1 4-3"x0.131" NAILS 4-3" 14GAGE STAPLES	FACE NAIL.
19. RAFTER TO PLATE (SEE SECT. 2308.10.1, TABLE 2308.10.1).	3-8d COMMON (2 1/2"x0.131") 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	TOENAIL.
23. BUILT-UP CORNER STUDS.	3-8d COMMON (2 1/2"x0.131") 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	TOENAIL.
24. BUILT-UP GIRDER AND BEAMS.	16d COMMON (3 1/2"x0.162") 3"x0.131" NAILS 3" 14GAGE STAPLES	24" oc 16" oc 16" oc
25. 2" PLANKS.	20d COM. (4"x0.192") 32" oc 3"x0.131" NAIL AT 24" oc 3" 14GAGE STAPLE AT 24" oc	FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES.
	2-20d COMMON (4"x0.192") 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	FACE NAIL AT ENDS AND AT EACH SPLICE.
26. COLLAR TIE TO RAFTER.	16d COMMON (3"x0.162")	AT EACH BEARING.
27. JACK RAFTER TO HIP.	3-10d COMMON (3"x0.148") 4-3"x0.131" NAILS 4-3" 14GAGE STAPLES	FACE NAIL.
	3-16d COMMON (3"x0.148") 4-3"x0.131" NAILS 4-3" 14GAGE STAPLES	TOENAIL.
28. ROOF RAFTER TO 2x RIDGE BEAM.	2-16d COMMON (3 1/2"x0.162") 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	FACE NAIL.
	2-16d COMMON (3 1/2"x0.162") 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	TOENAIL.
	2-16d COMMON (3 1/2"x0.162") 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	FACE NAIL.
	2-16d COMMON (3 1/2"x0.162") 3-3"x0.131" NAILS 3-3" 14GAGE STAPLES	FACE NAIL.

- a. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED.
- b. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16 INCH.



**PORTLAND
PARKS & RECREATION**

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Portland Parks and Recreation
Amanda Fritz, Commissioner - Kia Selley, Interim Director

GENERAL STRUCTURAL NOTES
Urban Forestry Maintenance Facility
PROJECT MANAGER: ROBIN LAUGHLIN
10910 N Denver, Portland, OR 97217

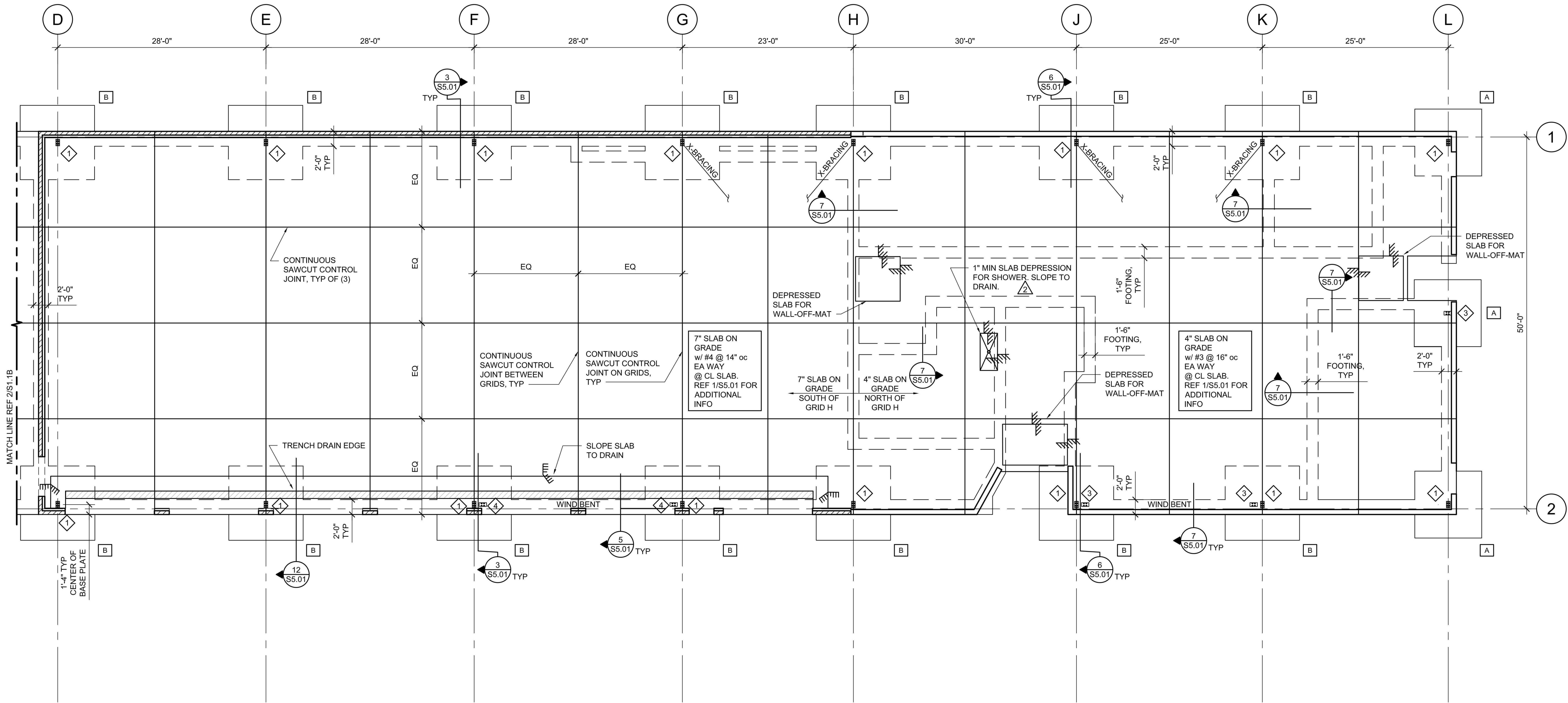
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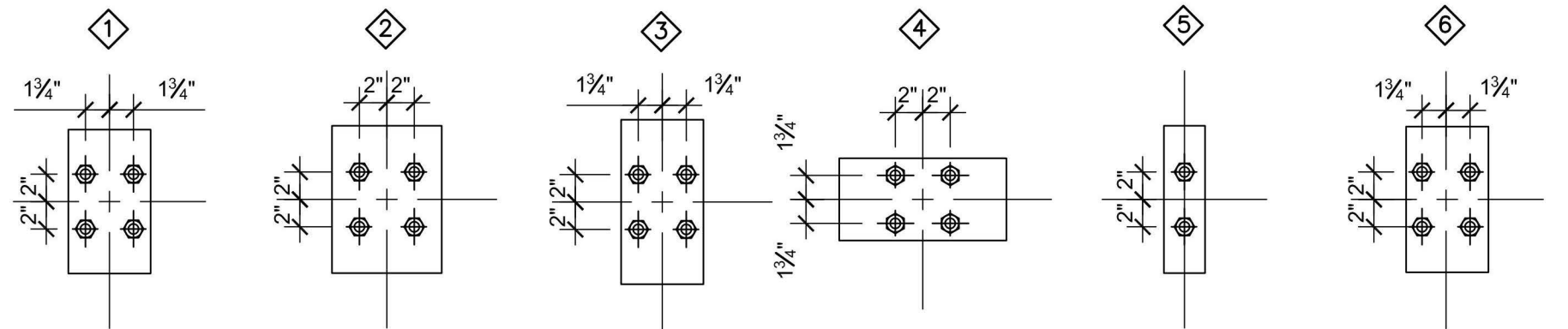
1 FOUNDATION PLAN - SECTOR A
1/8" = 1'-0"



DRAWING NOTES:

- REFERENCE ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND DIMENSIONS NOT SHOWN. STEEL COLUMN DIMENSIONS INDICATED ON THE PLAN ARE TO BOLT GROUP CENTERLINE. SPECIFIED FOOTINGS ARE CENTERED ON COLUMN BOLT GROUPS.
-  INDICATES FOOTING TYPE. REFERENCE THE SCHEDULE ON THIS SHEET.
- REFERENCE THE STRUCTURAL GENERAL NOTES FOR SUBGRADE PREPARATION REQUIREMENTS.
- FS INDICATES APPROXIMATE LOCATION OF FOOTING STEP PER DETAIL 2/S5.01. COORDINATE STEP LOCATIONS w/ SITE GRADE.
-  INDICATES BASIS OF DESIGN METAL BUILDING ANCHOR BOLT PATTERN. REFERENCE S1.1A FOR BASIS OF DESIGN. BOLT LAYOUT, COORDINATE FINAL CONFIGURATION w/ FINAL METAL BUILDING DESIGN.

SPREAD FOOTING SCHEDULE				
FOOTING MARK	WIDTH x LENGTH	THICK-NESS	BOTTOM BARS	TOP BARS
A	9'-0" x 9'-0"	2'-0"	#6 @ 10" oc BOT EA WAY	N/A
B	10'-0" x 10'-0"	2'-0"	#6 @ 10" oc BOT EA WAY	N/A

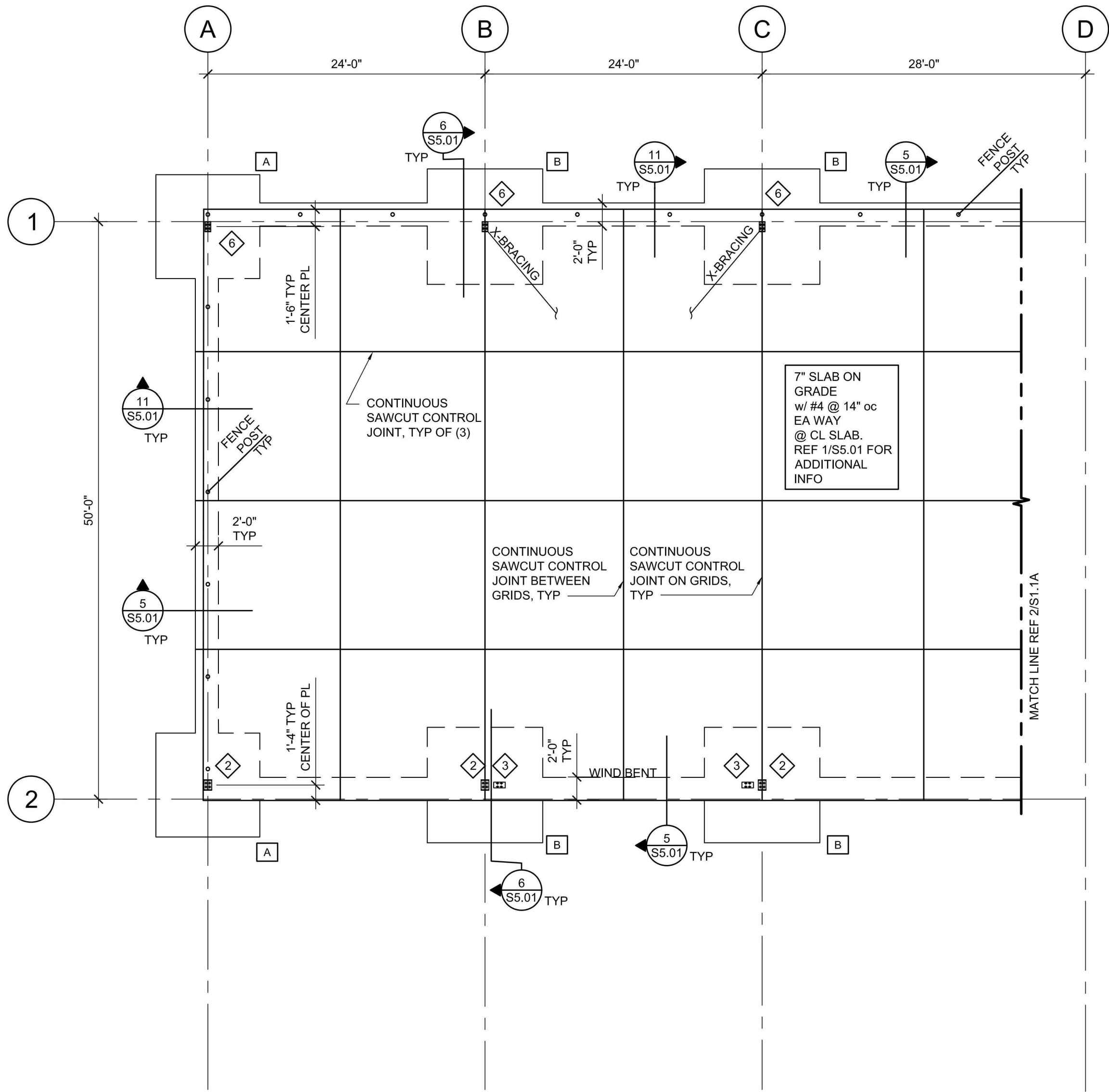


BASIS OF DESIGN ANCHOR BOLT CONFIGURATION

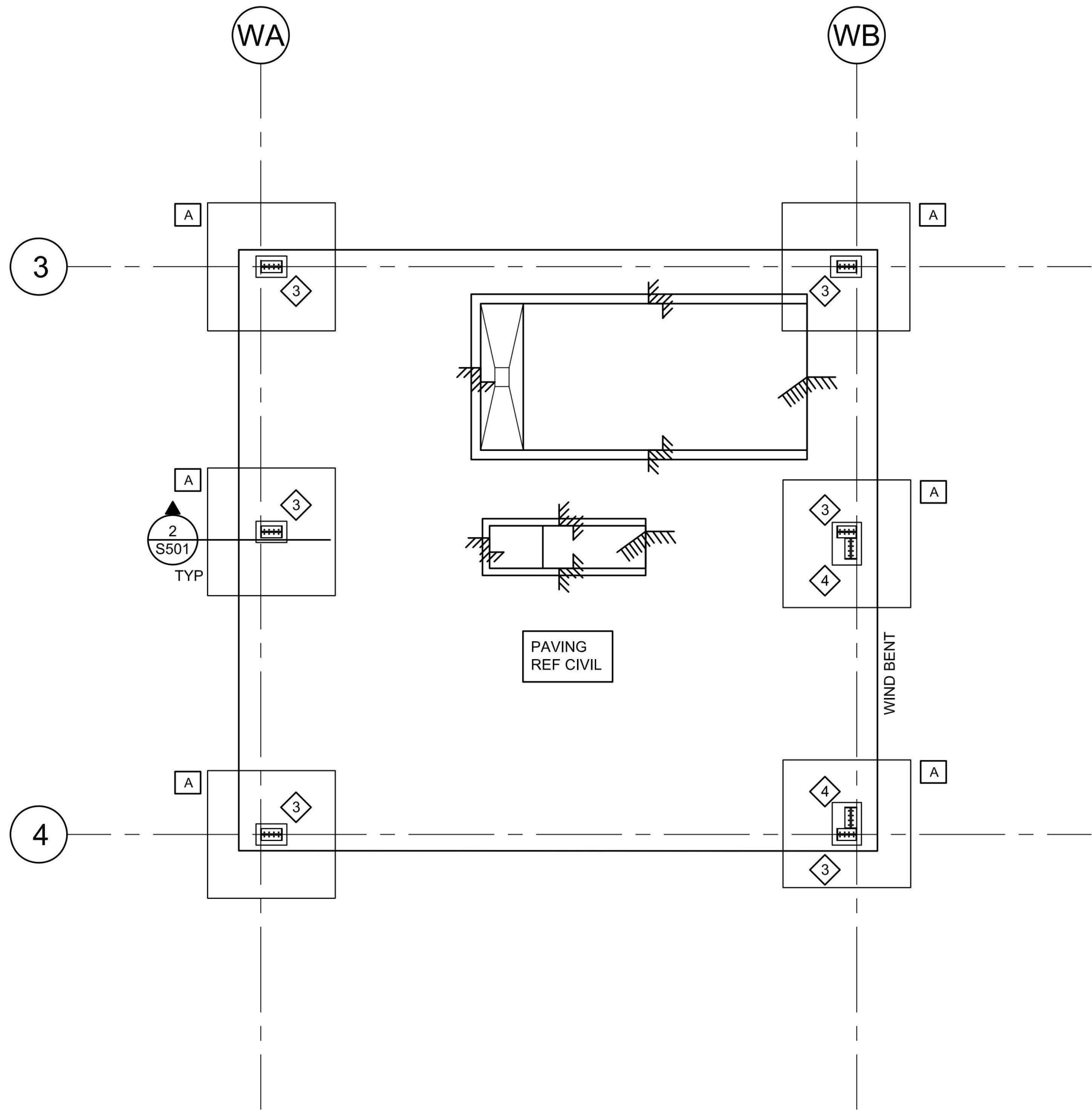
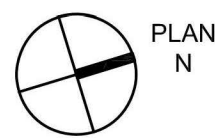
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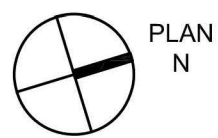






1 FOUNDATION PLAN - SECTOR B
1/8" = 1'-0"

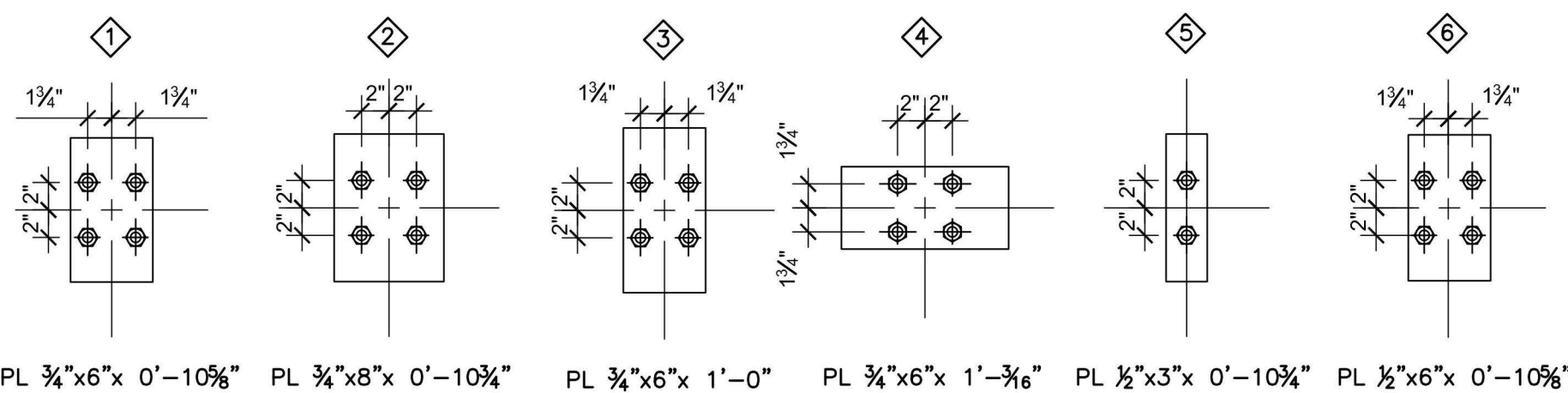


2 FOUNDATION PLAN - WASH RACK & TRASH ENCLOSURE
1/8" = 1'-0"



DRAWING NOTES:

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-  INDICATES BASIS OF DESIGN METAL BUILDING ANCHOR BOLT PATTERN. REFERENCE S1.1A FOR BASIS OF DESIGN. BOLT LAYOUT, COORDINATE FINAL CONFIGURATION w/ FINAL METAL BUILDING DESIGN.



PL 3/4"x6"x 0'-10 5/8" PL 3/4"x8"x 0'-10 3/4" PL 3/4"x6"x 1'-0" PL 3/4"x6"x 1'-3/8" PL 1/2"x3"x 0'-10 3/4" PL 1/2"x6"x 0'-10 5/8"

BASIS OF DESIGN ANCHOR BOLT CONFIGURATION

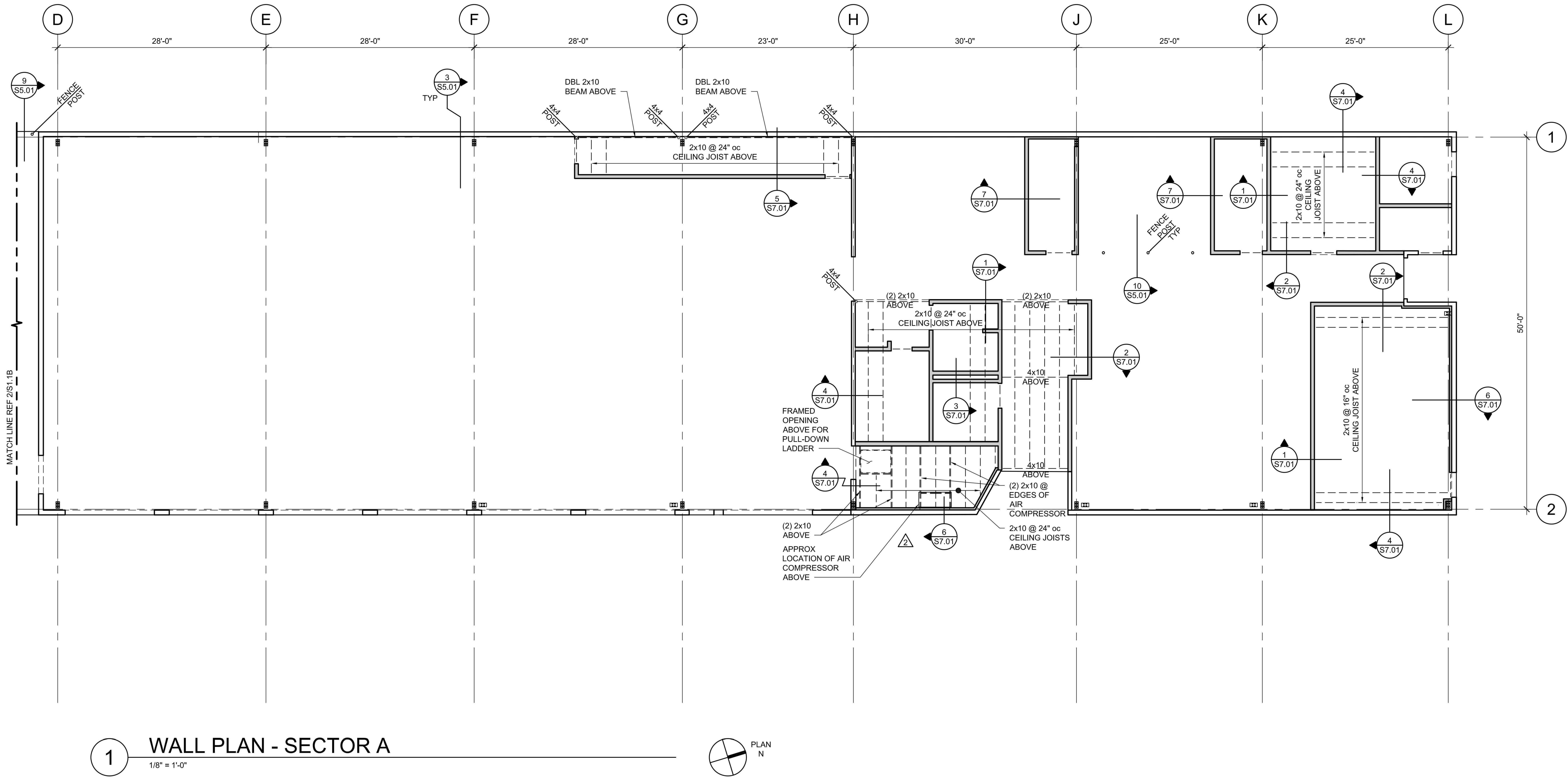
SPREAD FOOTING SCHEDULE				
FOOTING MARK	WIDTH x LENGTH	THICK-NESS	BOTTOM BARS	TOP BARS
A	9'-0" x 9'-0"	2'-0"	#6 @ 10" oc BOT EA WAY	N/A
B	10'-0" x 10'-0"	2'-0"	#6 @ 10" oc BOT EA WAY	N/A



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1 WALL PLAN - SECTOR A
1/8" = 1'-0"

Portland Parks and Recreation

Amanda Fritz, Commissioner - Kia Selley, Interim Director

**WALL PLAN - SECTOR A
Urban Forestry Maintenance Facility**

10910 N Denver, Portland, OR 97217
PROJECT MANAGER: ROBIN LAUGHLIN

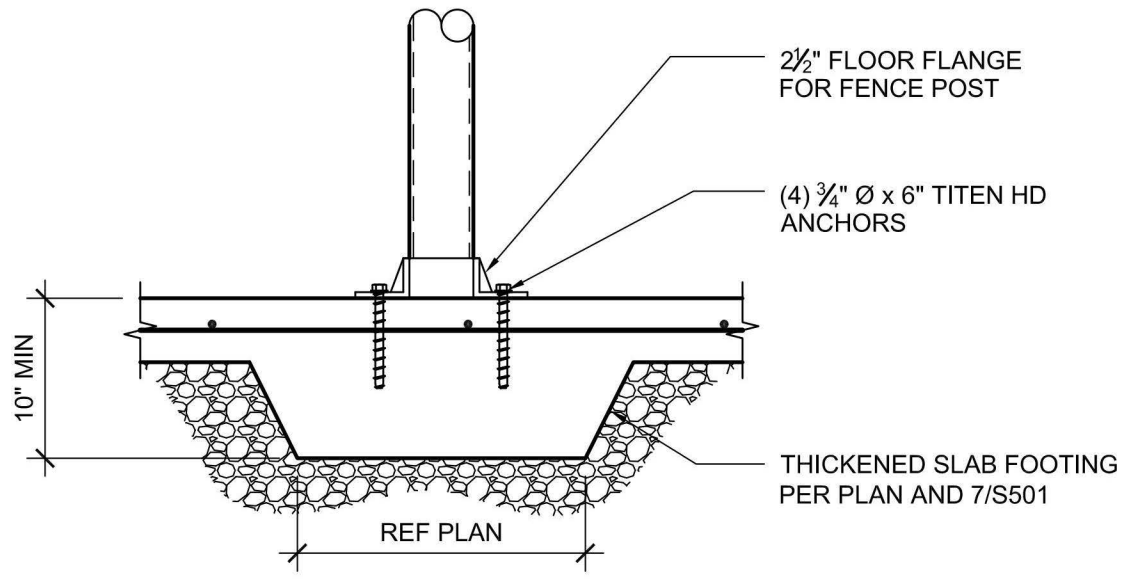
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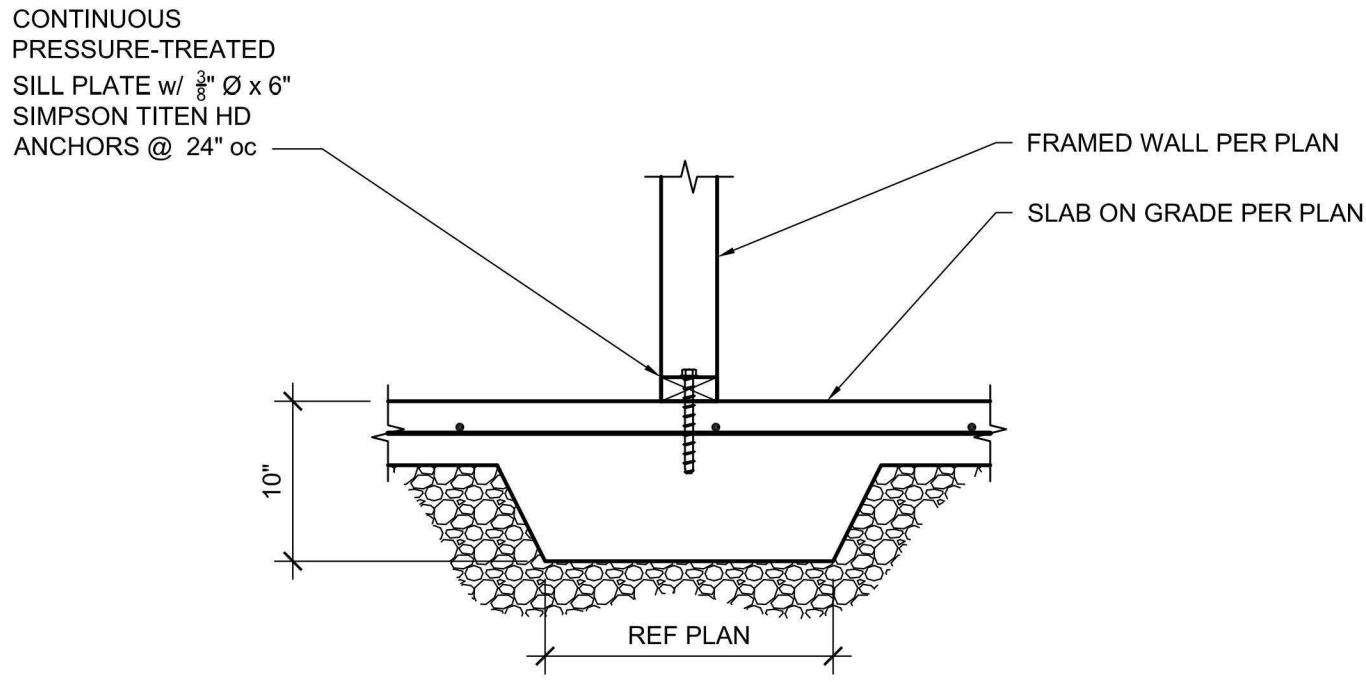
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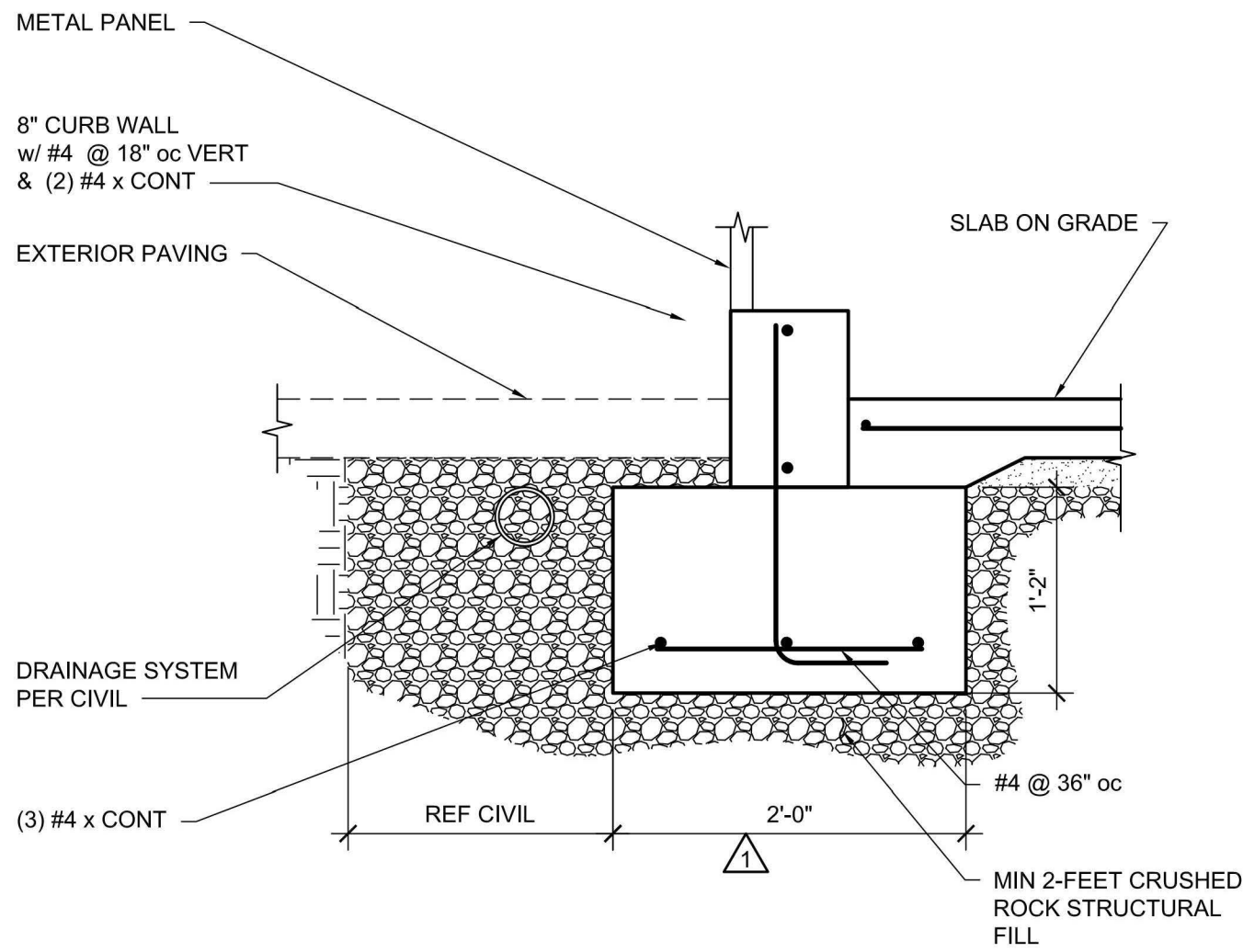
INTERIOR FENCE POST BASE DETAIL

10
1" = 1'-0"



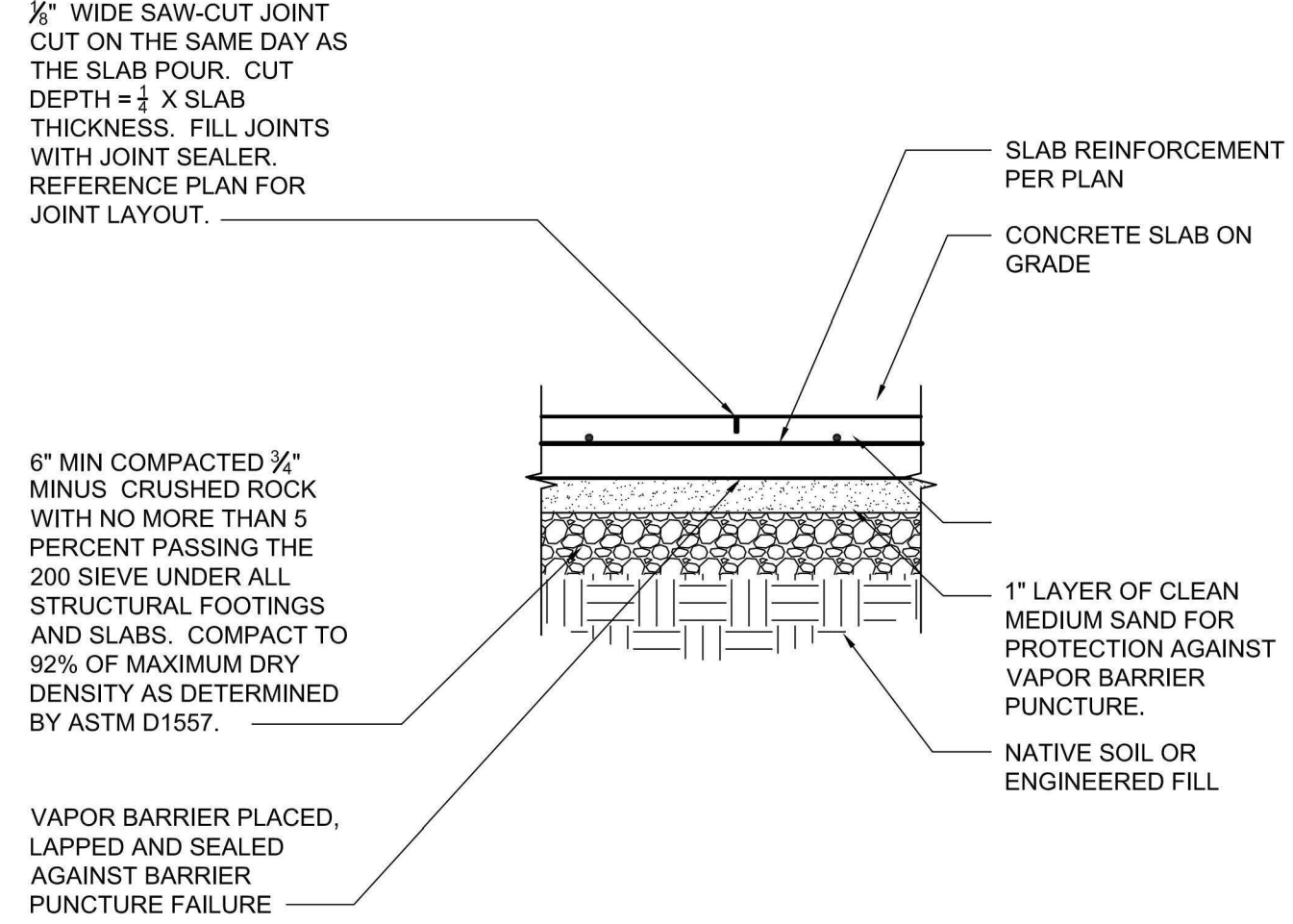
LOAD BEARING WALL DETAIL

7
1" = 1'-0"



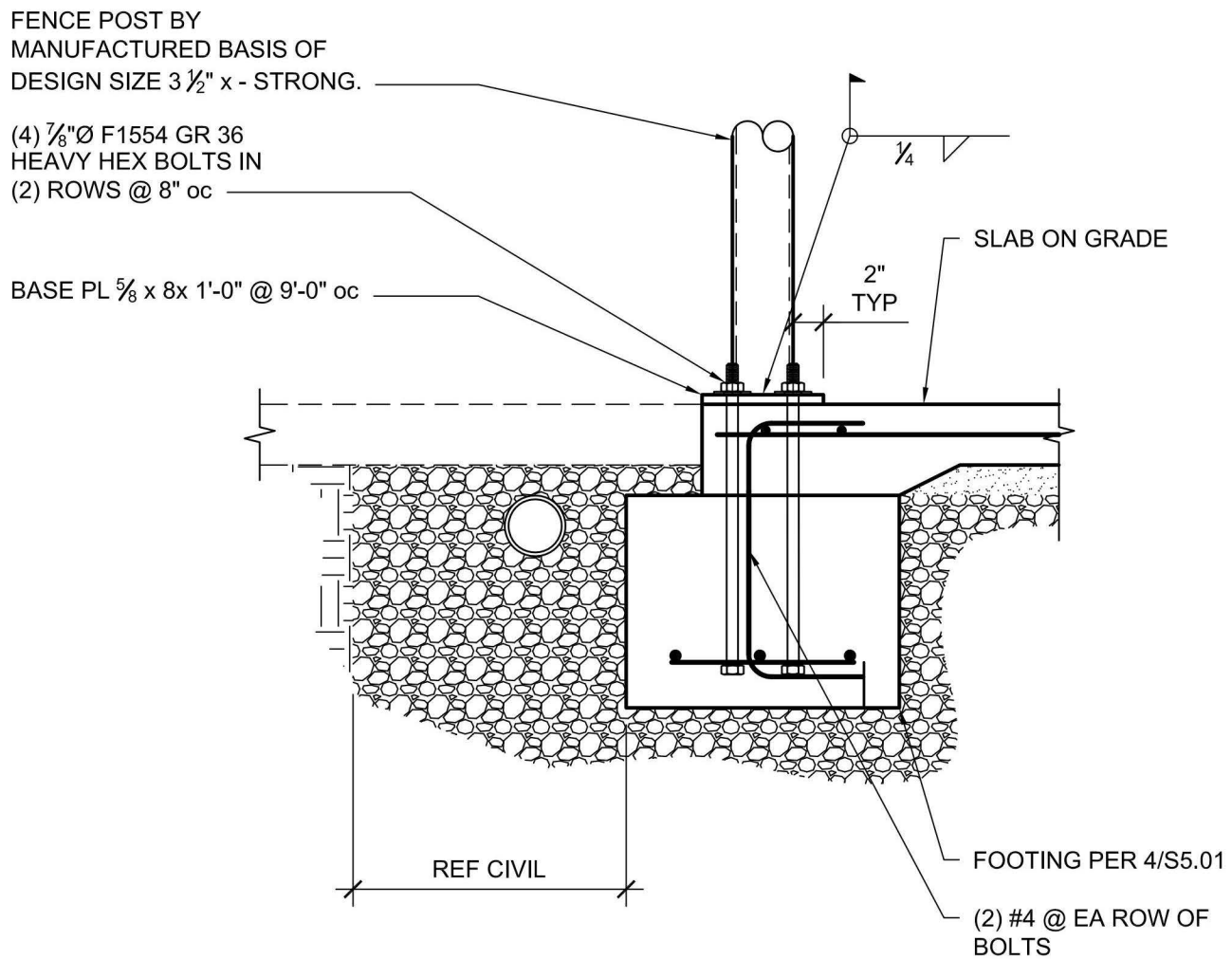
STEM WALL AND FOOTING DETAIL

4
1" = 1'-0"



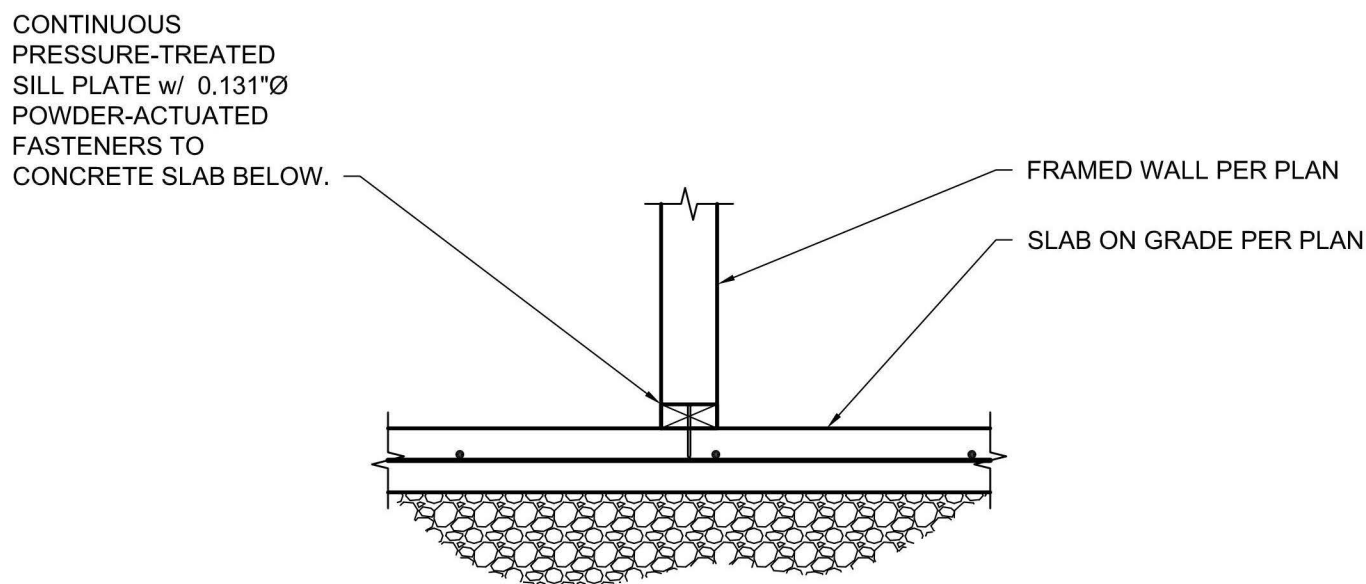
TYPICAL SLAB ON GRADE

1
1" = 1'-0"



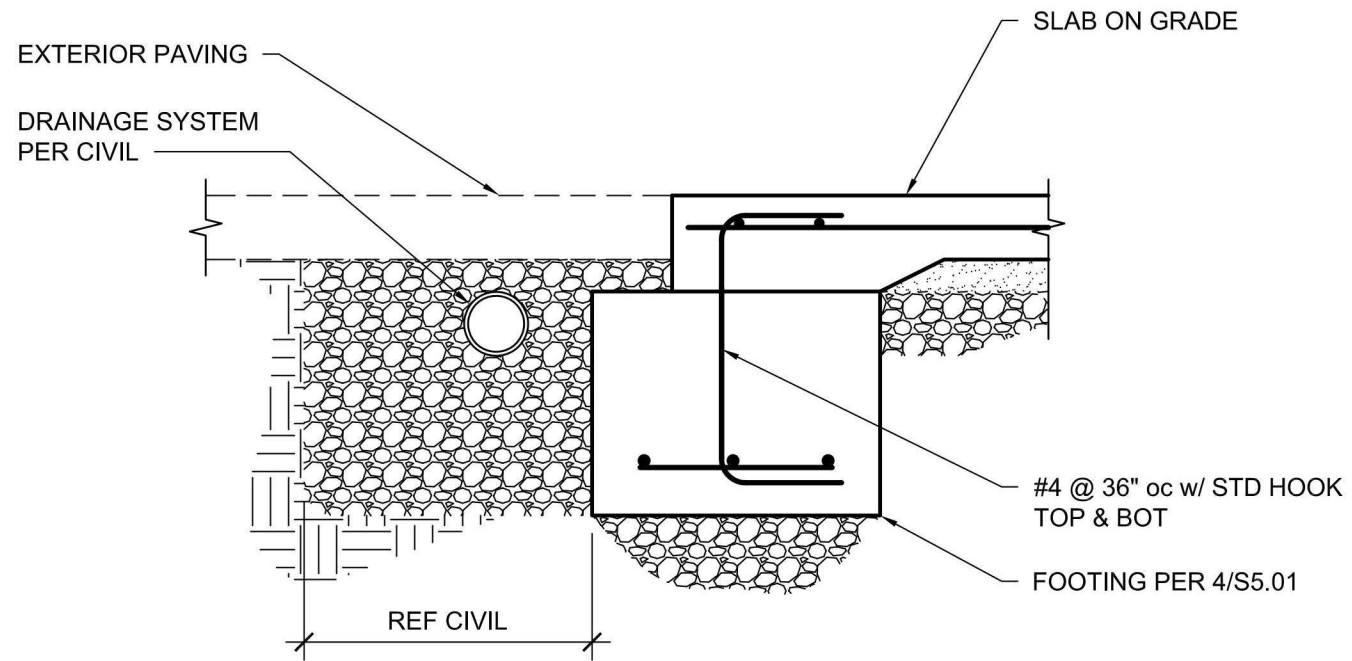
EXTERIOR FENCE POST BASE DETAIL

11
1" = 1'-0"



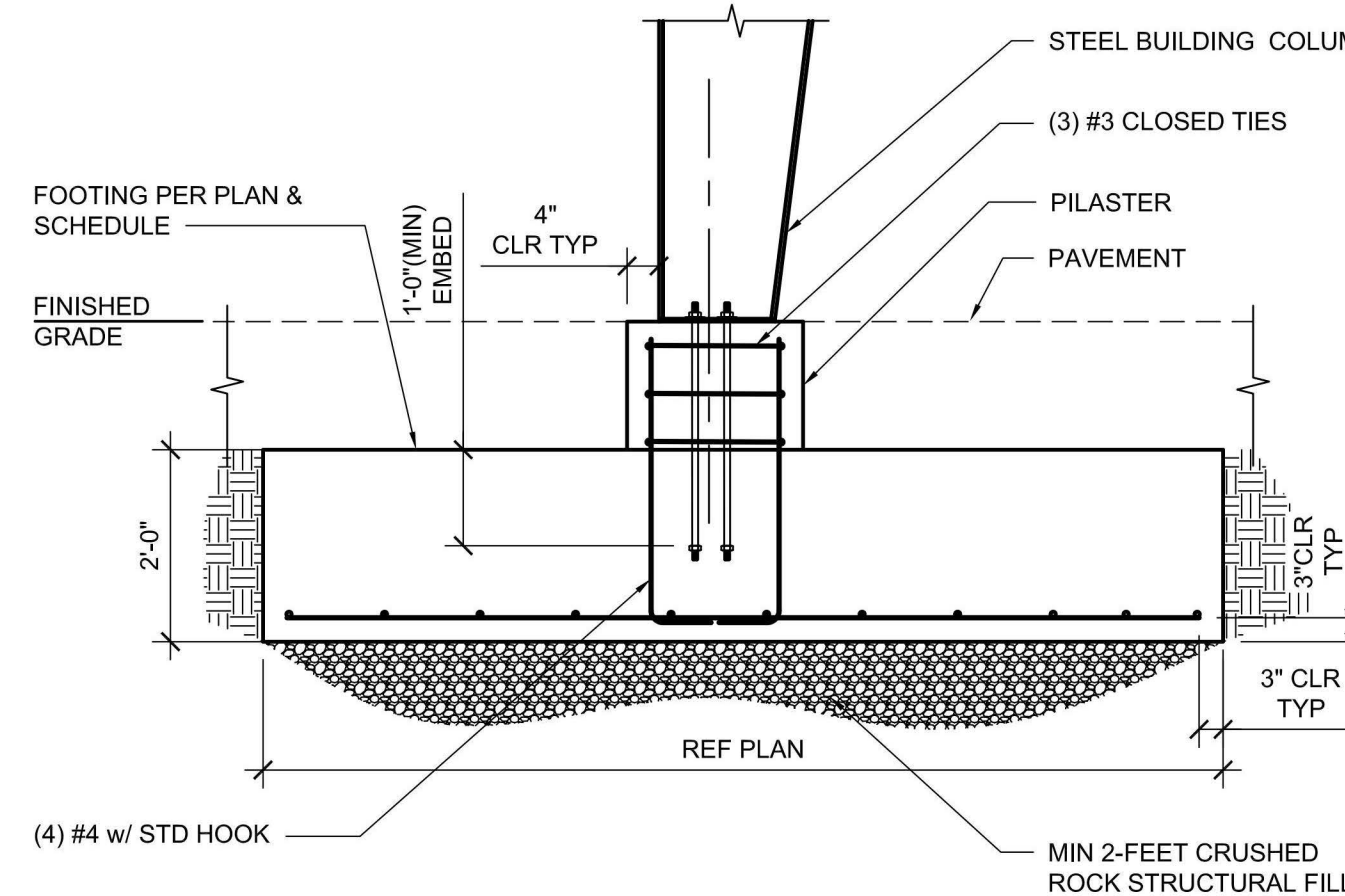
PARTITION WALL DETAIL

8
1" = 1'-0"



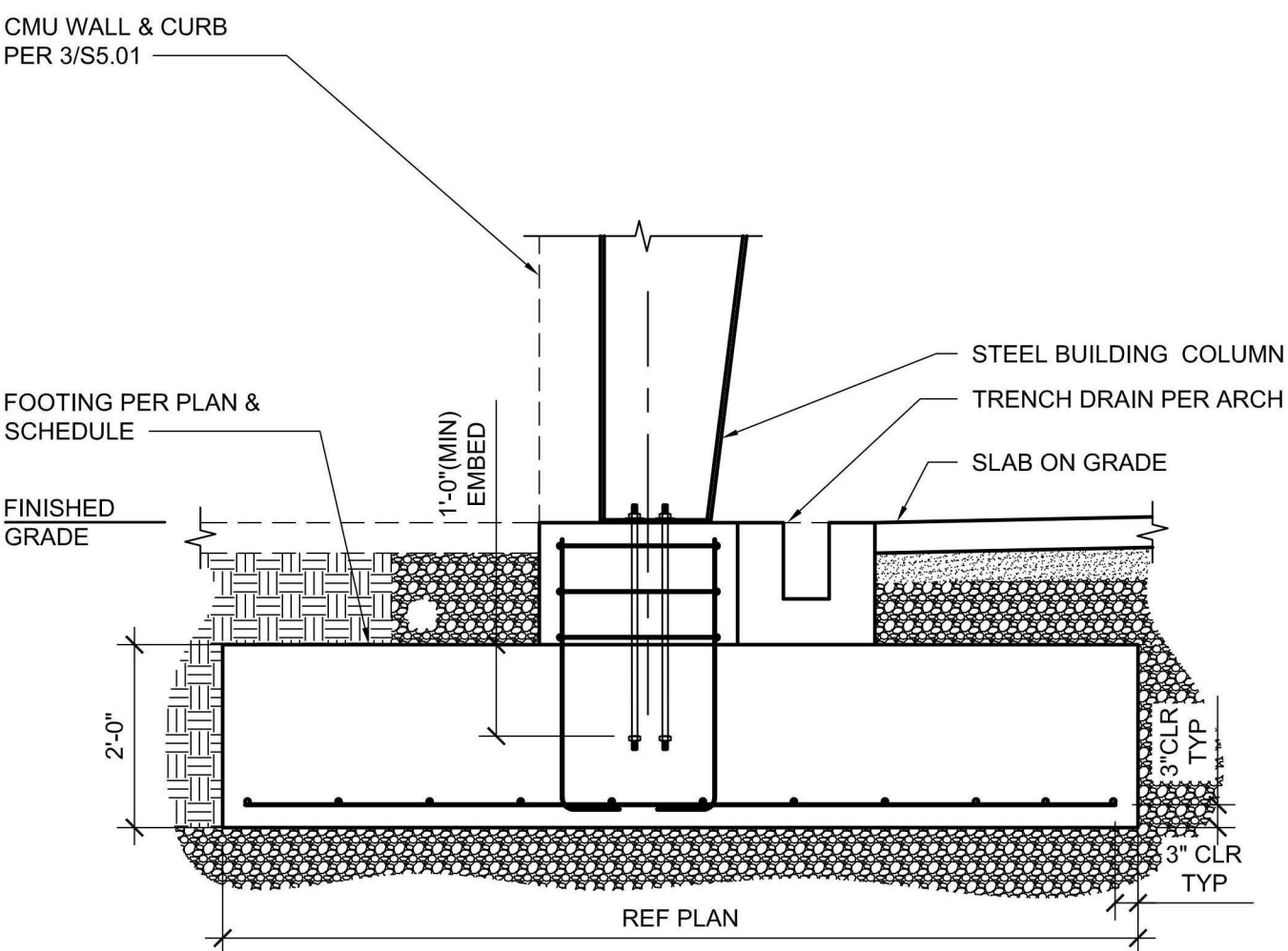
SLAB EDGE

5
1" = 1'-0"



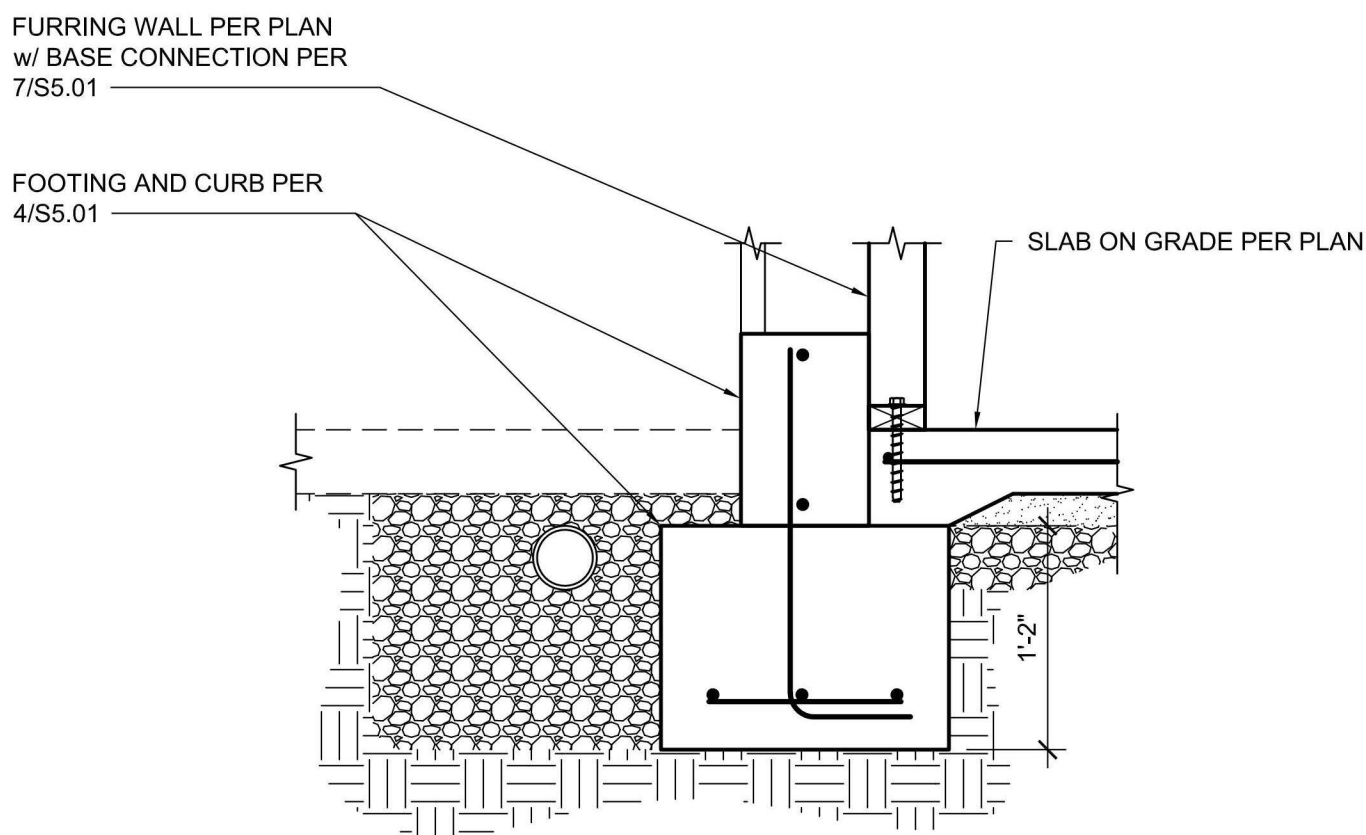
FRAME SECTION AT PILASTER

2
1/2" = 1'-0"



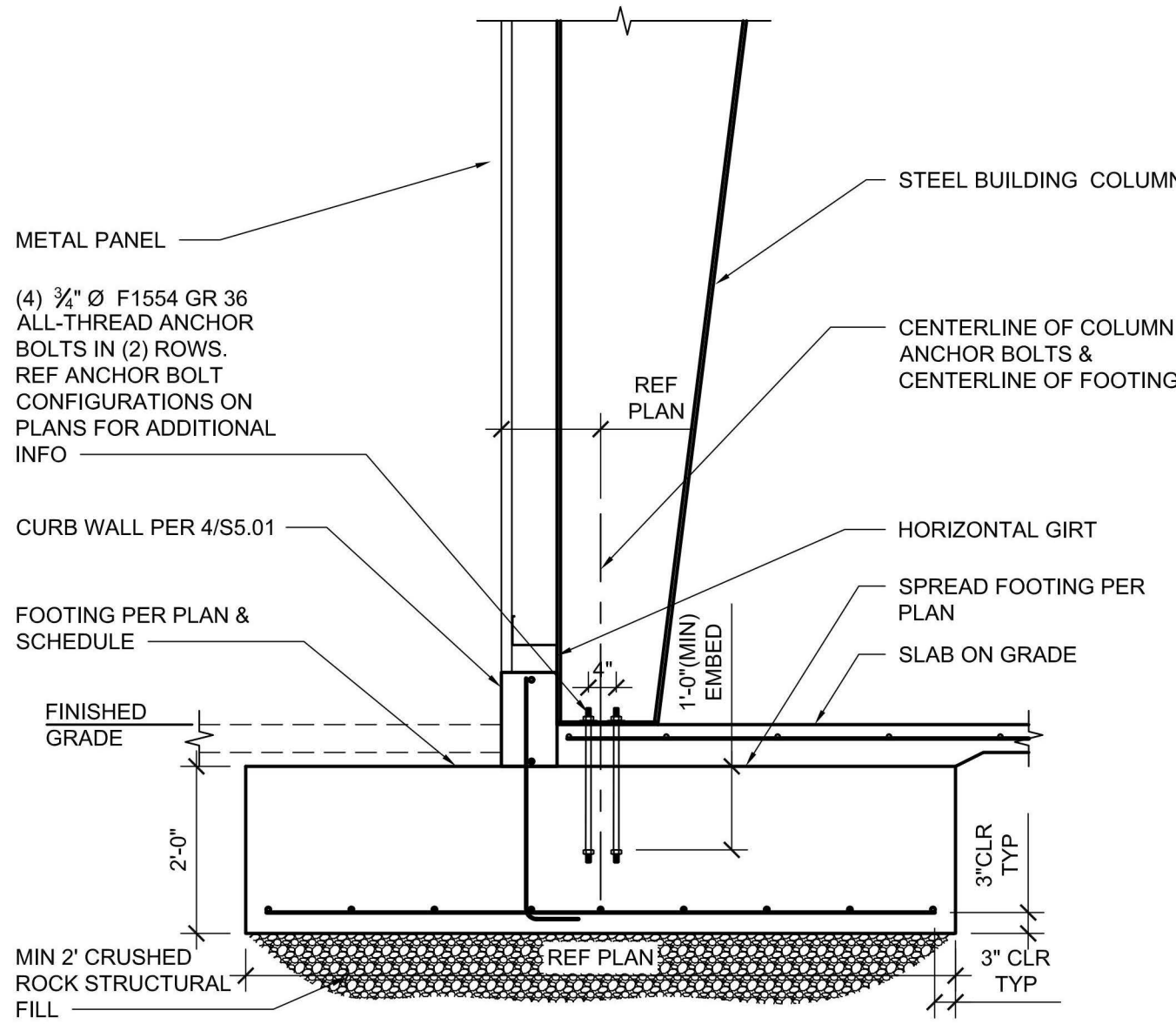
FRAME SECTION AT CURB WALL

12
1/2" = 1'-0"



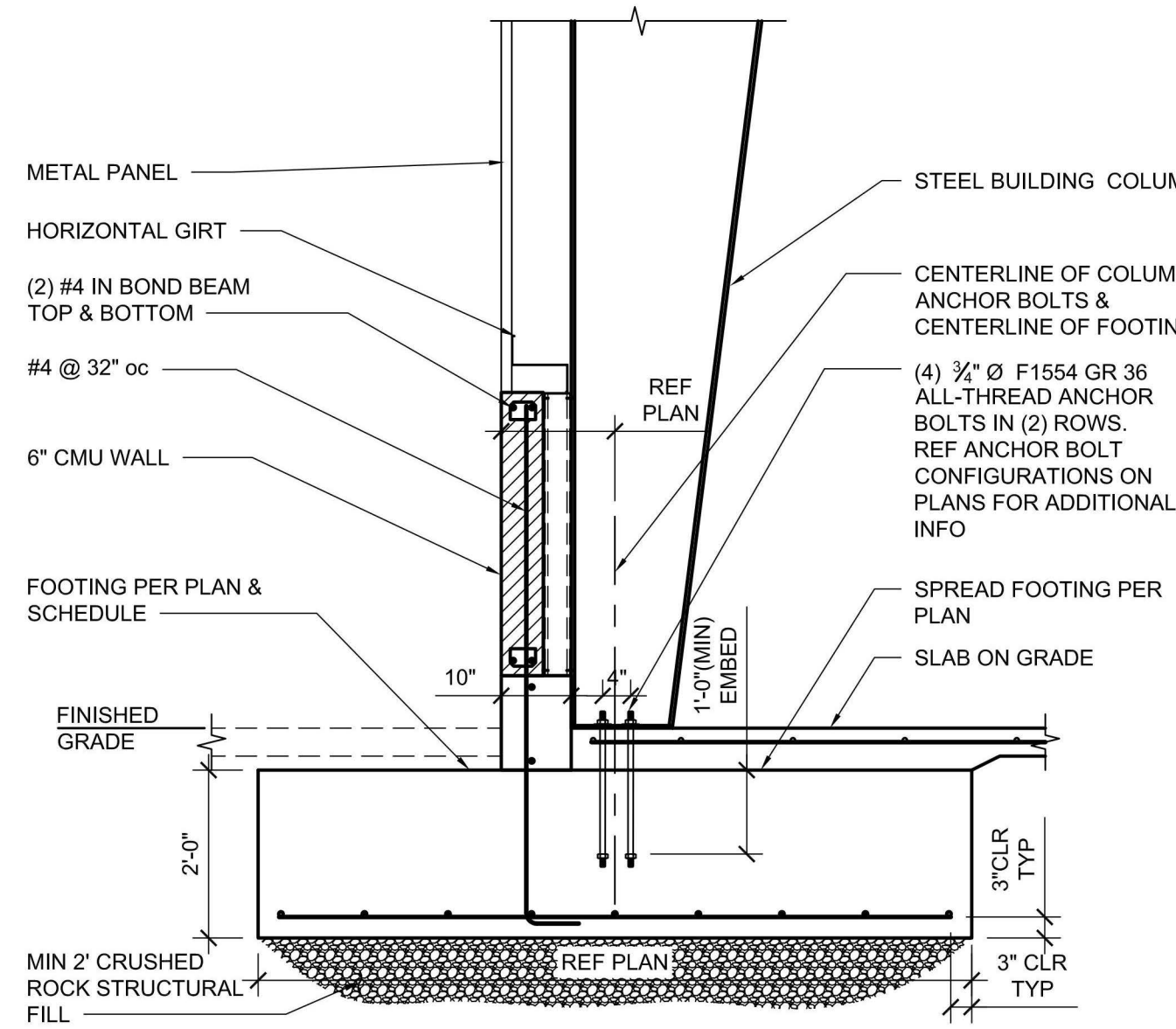
FURRING WALL DETAIL

9
1" = 1'-0"



FRAME SECTION AT CURB WALL

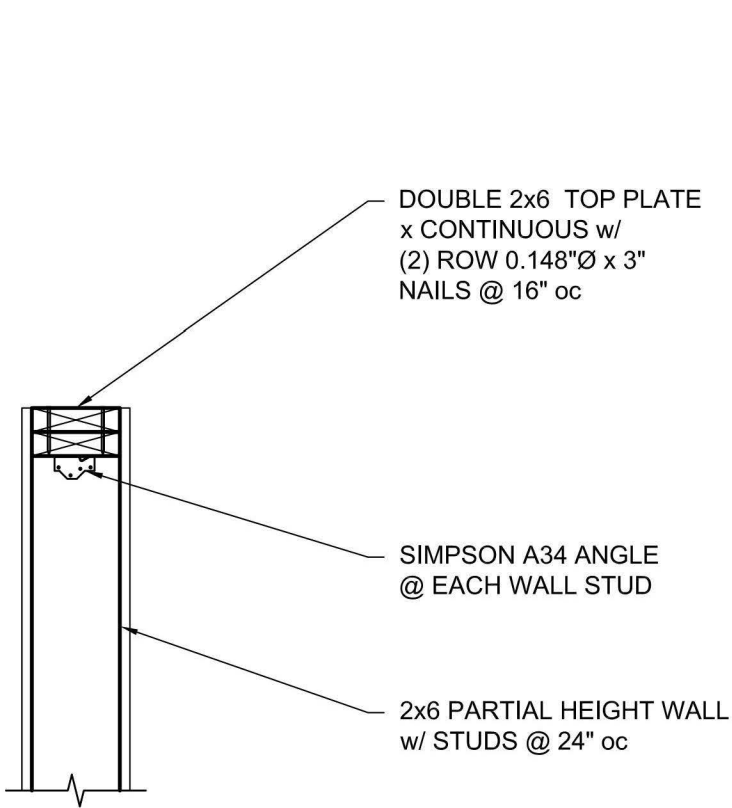
6
1/2" = 1'-0"



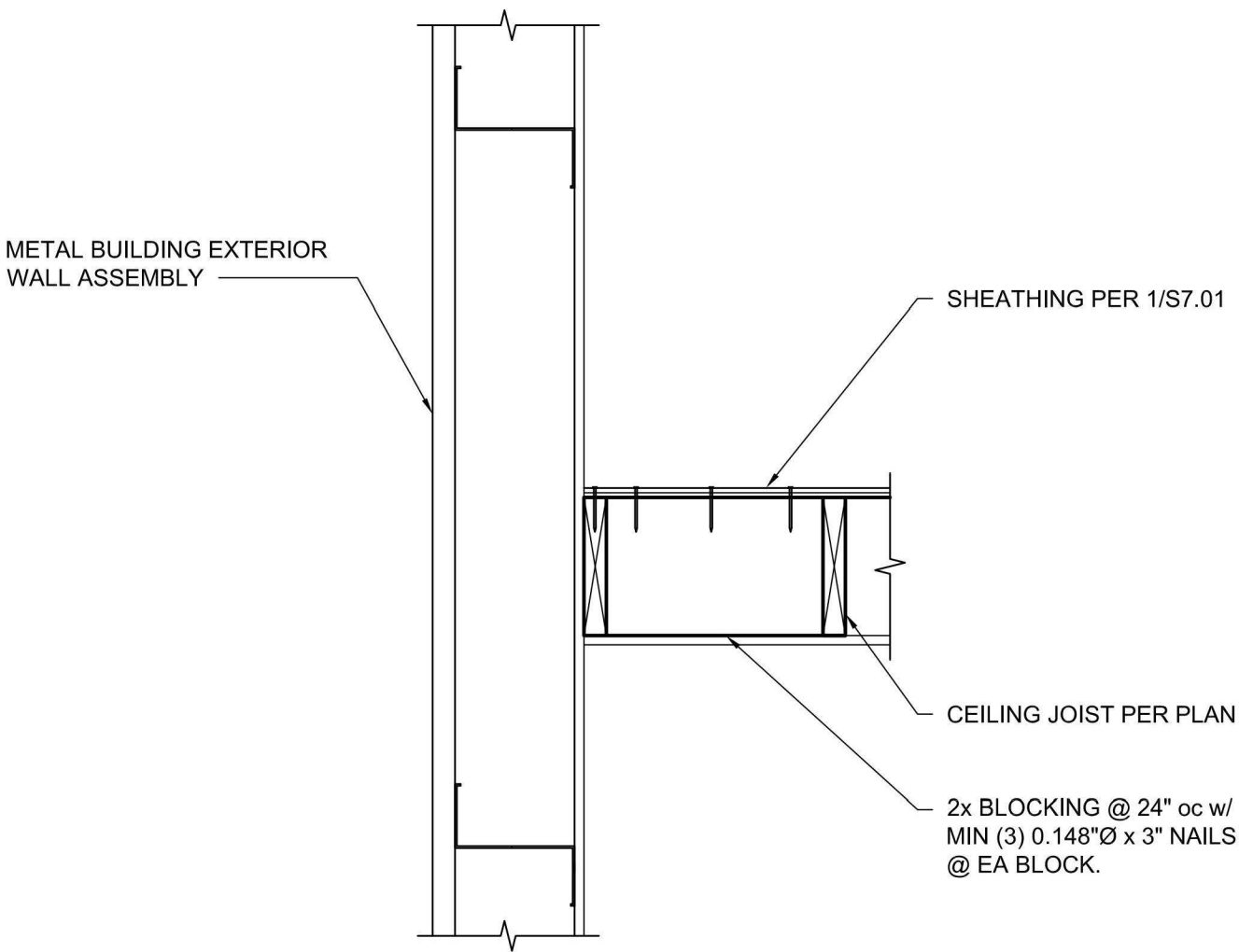
TYPICAL FRAME SECTION

3
1/2" = 1'-0"

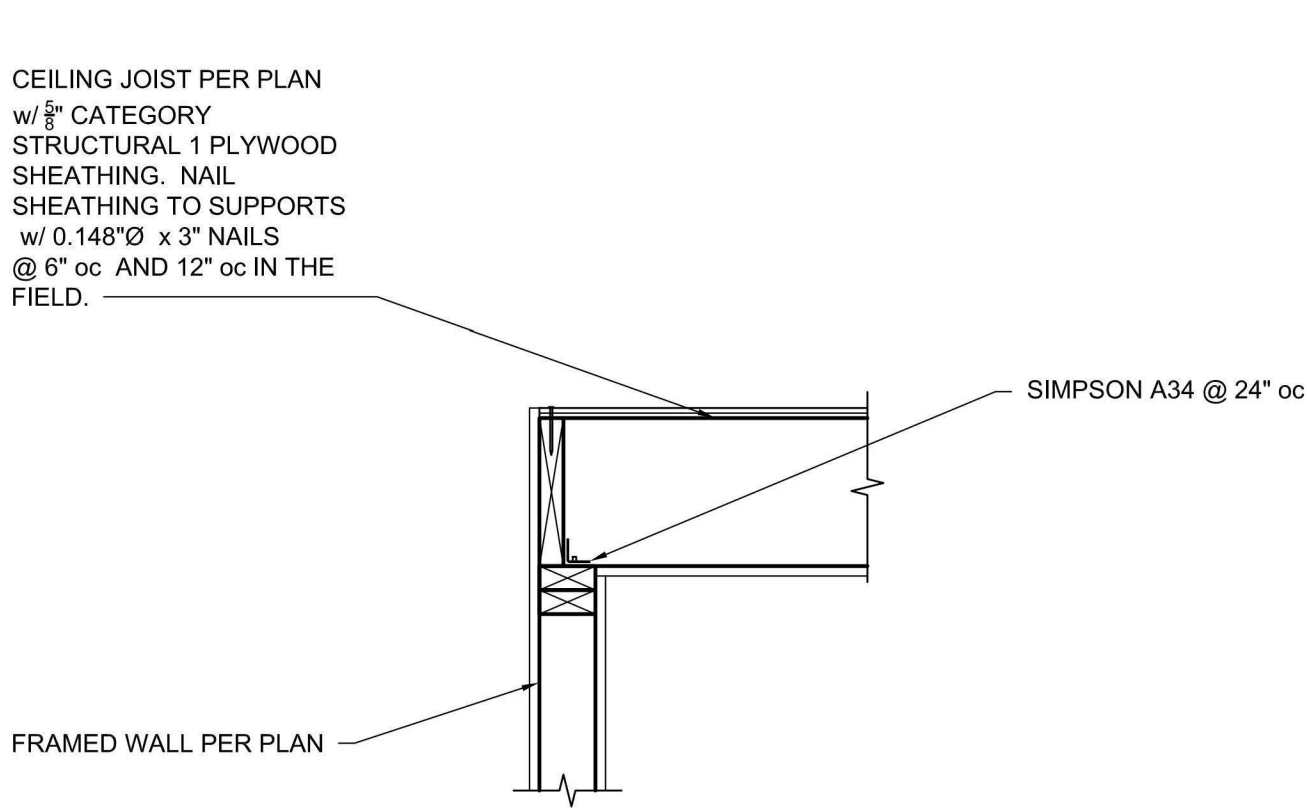
REVISIONS		
DATE	DESCRIPTION	BY
7-23-19	PLAN REVIEW	TB
1-15-19	SD CHECKSHEET	TB



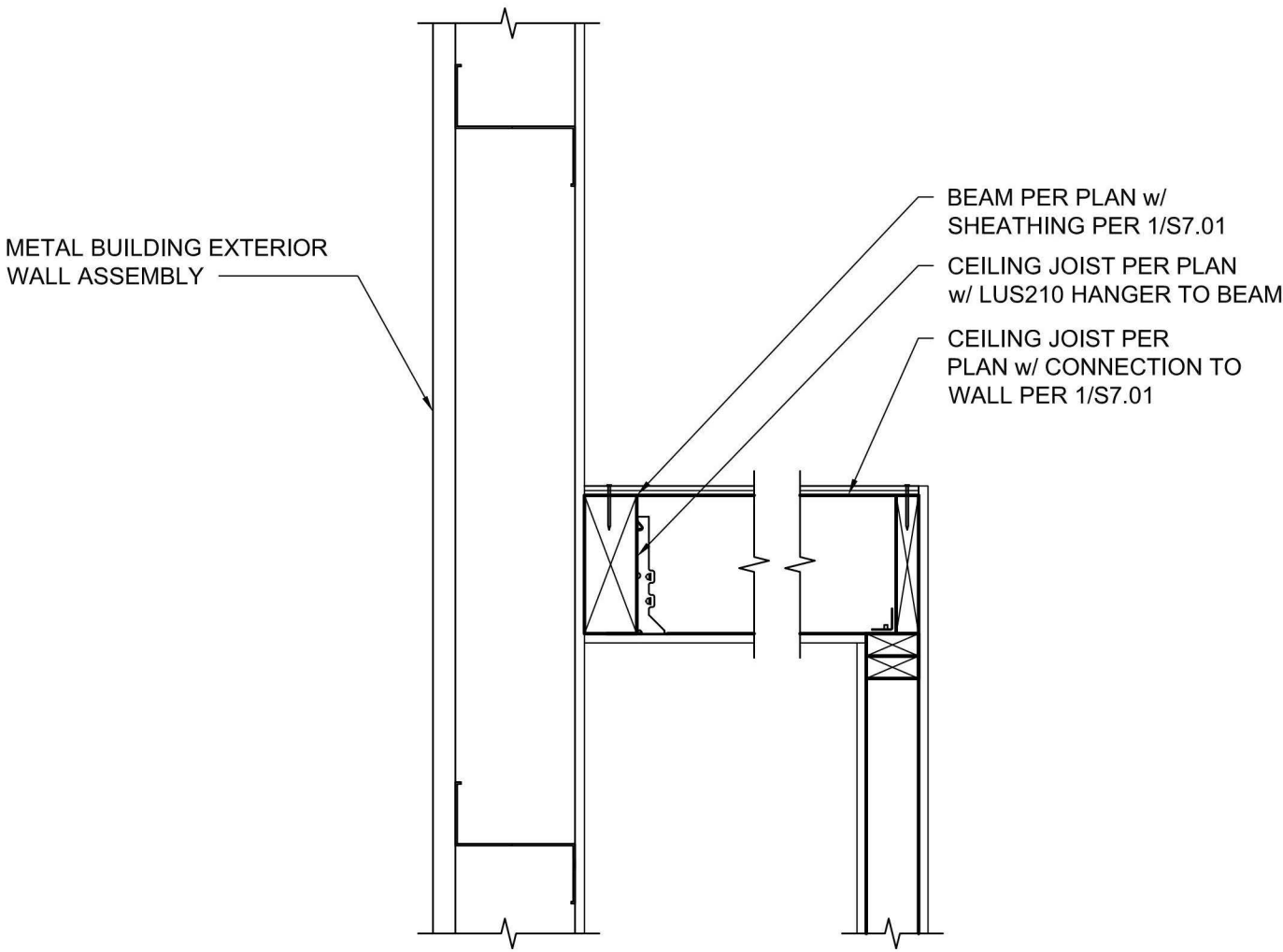
7 PARTIAL HEIGHT PARTITION DETAIL
1" = 1'-0"



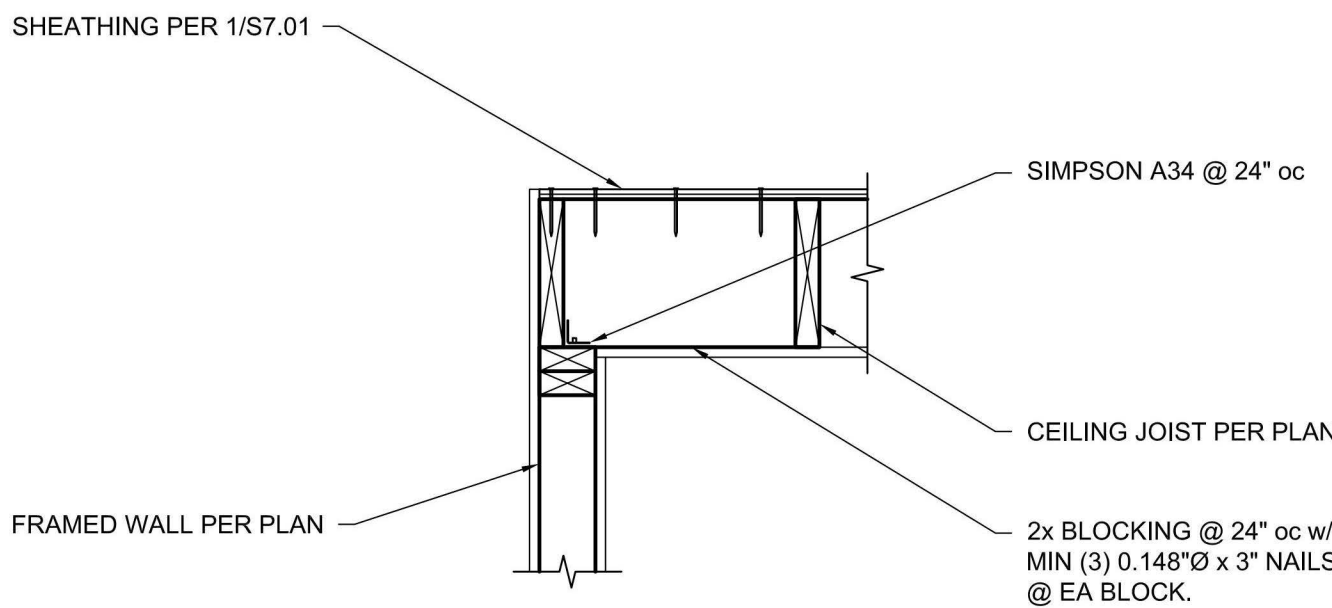
4 FRAMED WALL DETAIL
1" = 1'-0"



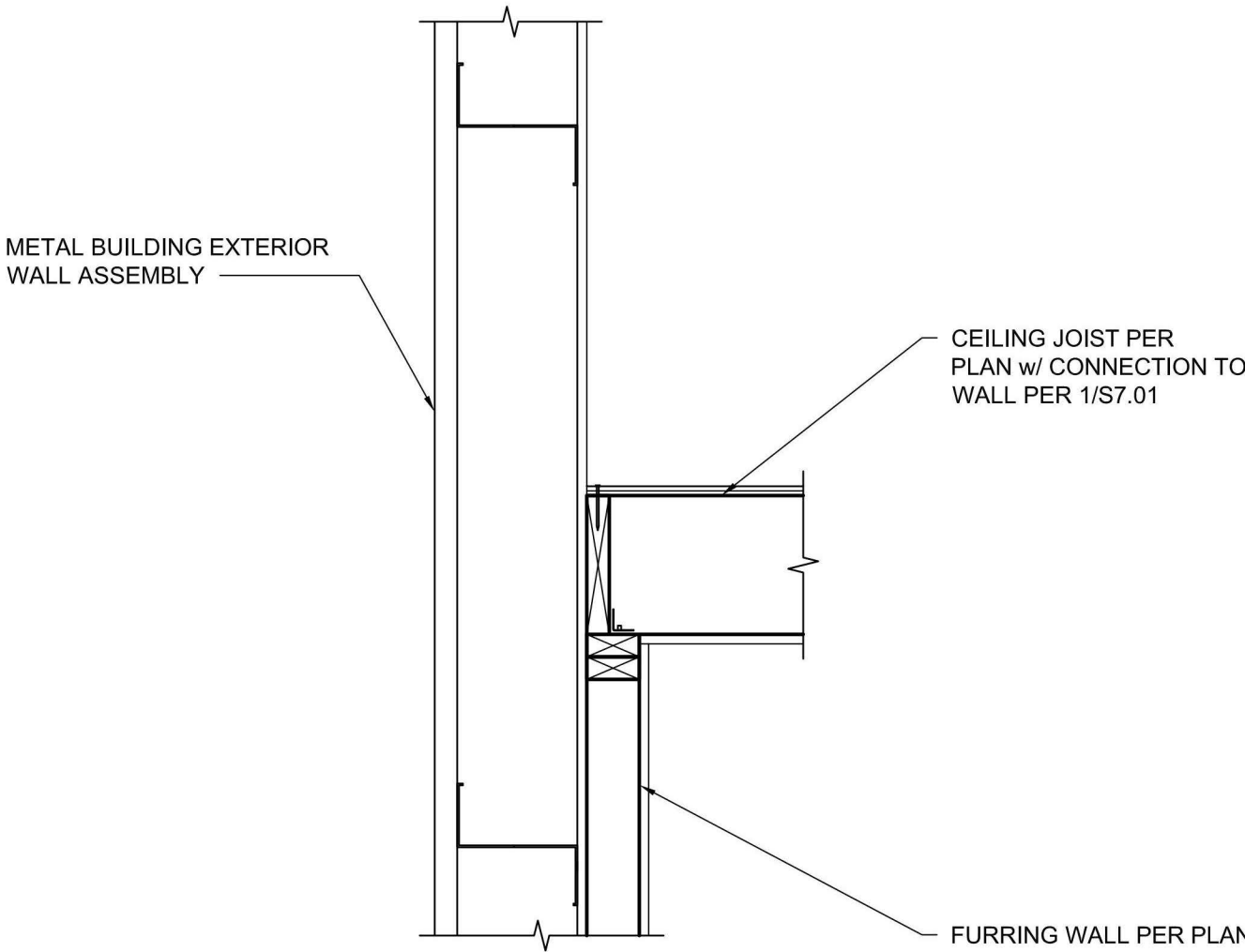
1 FRAMED WALL DETAIL
1" = 1'-0"



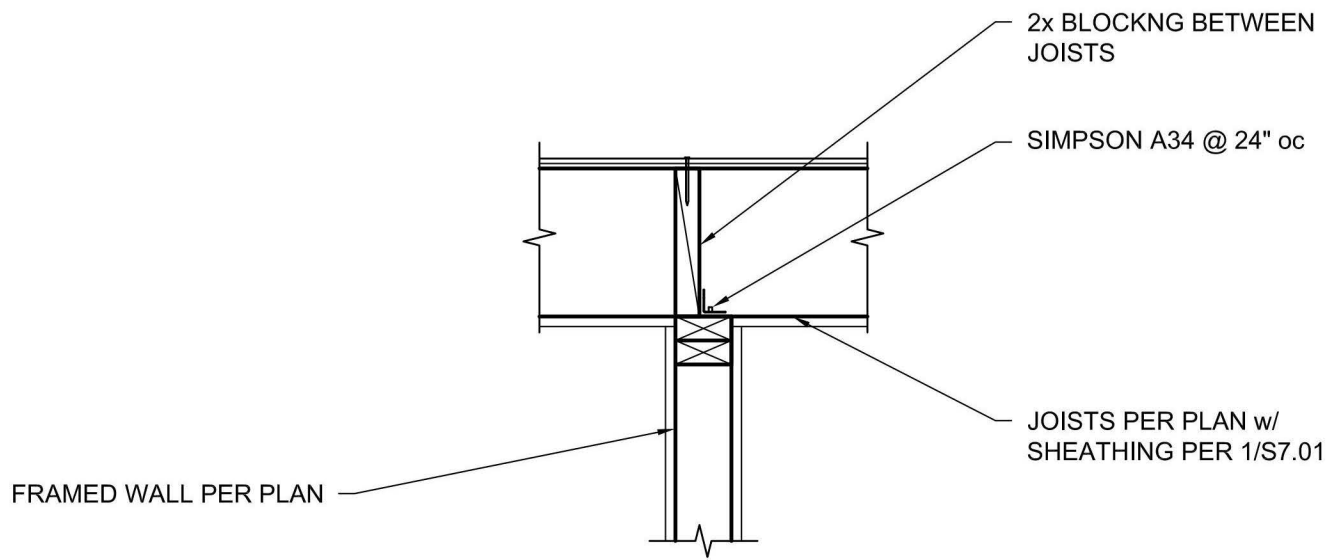
5 FRAMED WALL DETAIL
1" = 1'-0"



2 FRAMED WALL DETAIL
1" = 1'-0"



6 FRAMED WALL DETAIL
1" = 1'-0"



3 FRAMED WALL DETAIL
1" = 1'-0"

REVISIONS		
DATE	DESCRIPTION	BY
7-23-19	PLAN REVIEW	TB
1-15-19	SD CHECKSHEET	TB

