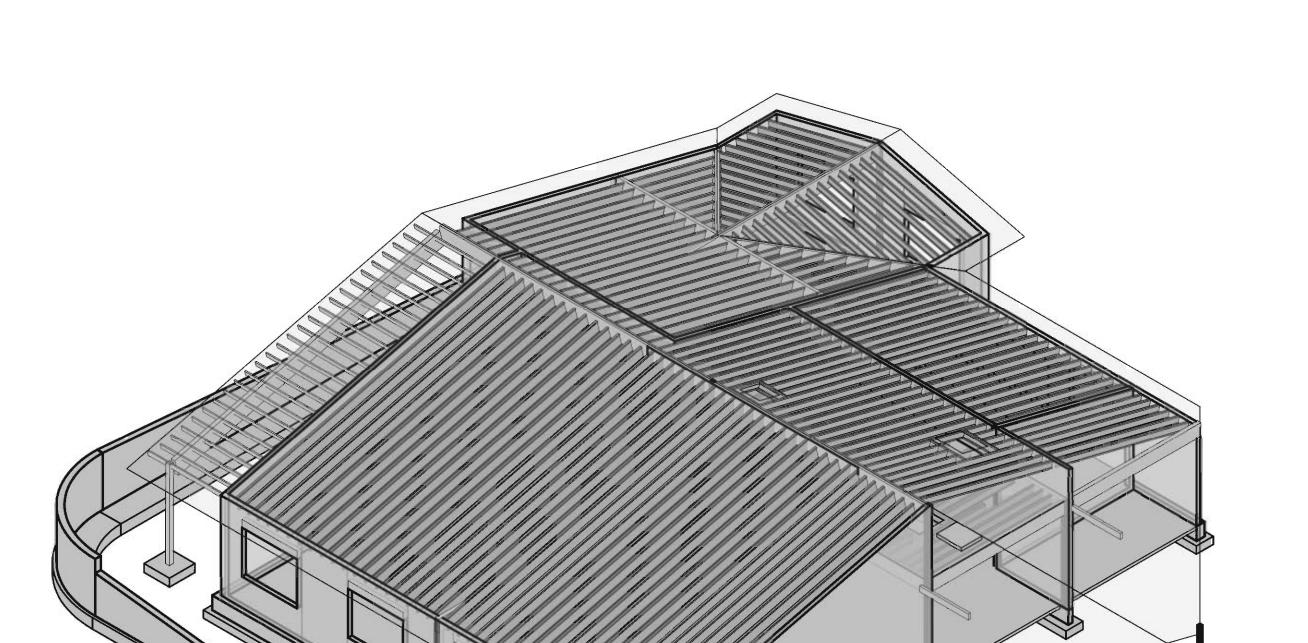
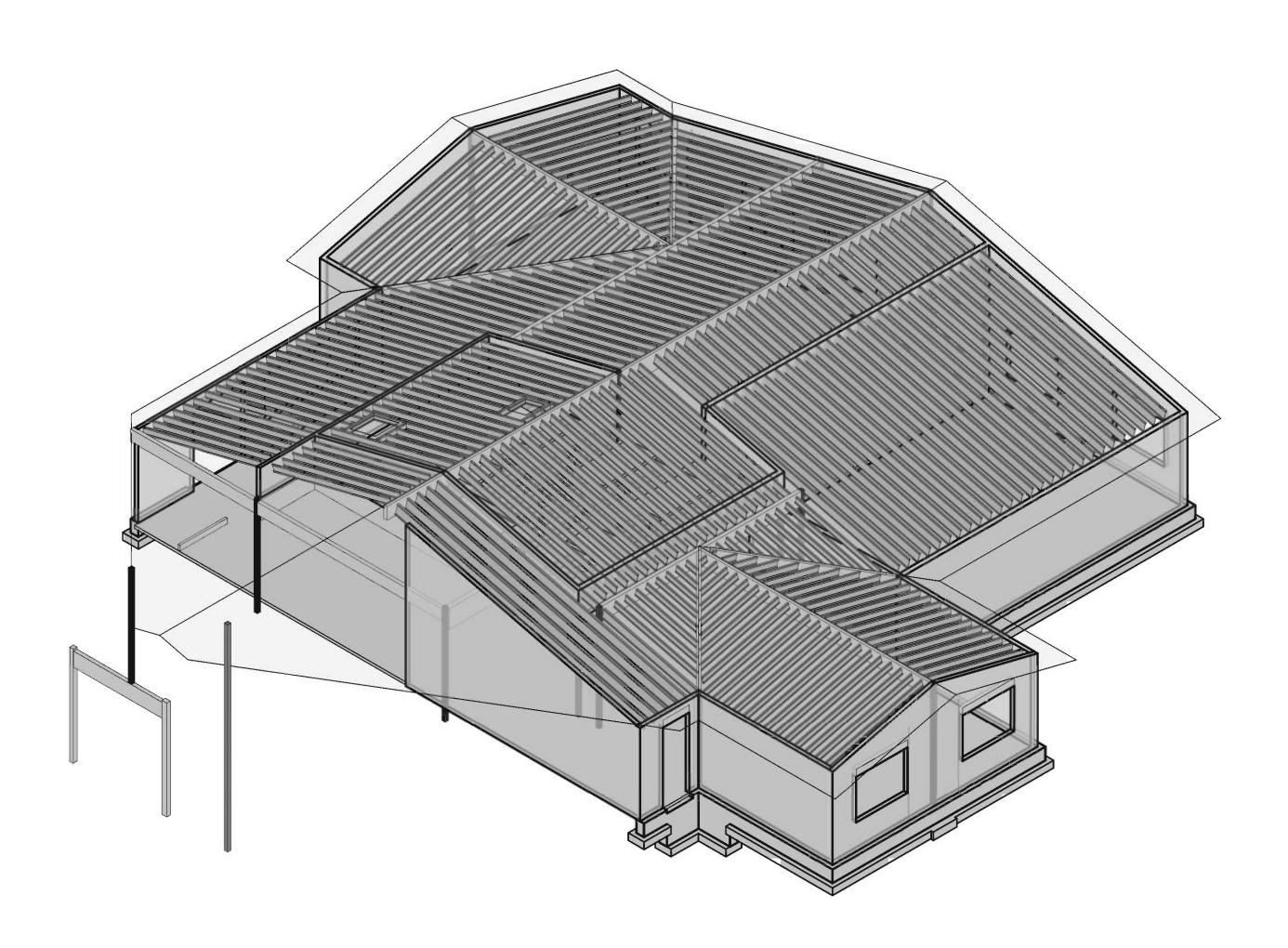
Jurisdiction Stamp Area



WEST ADDITION



NORTH ADDITION



71 Columbia Street, Suite 500 Seattle, Washington 98104 T (206) 340 9500 F (206) 340 9519

CIVIL ENGINEER AND LANDSCAPE MacKay Sposito 1325 SE Tech Center Drive, Suite 140 Vancouver, WA 98683 T (360) 695 3411

STRUCTURAL ENGINEER PCS Structural Solutions One Main Place 101 SW Main Street, Suite 280 Portland, OR 97204

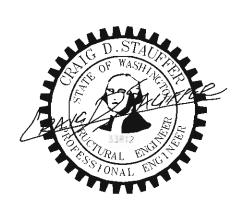
T (503) 232 3746 MECHANICAL ENGINEER BCE Engineers 6021 12th St E, Suite 200 Fife, WA 98424 T (253) 922-0446

ELECTRICAL ENGINEER BCE Engineers 6021 12th St E, Suite 200 Fife, WA 98424 T (253) 922-0446

ACOUSTIC Stantec 4100 194th St., SW, Ste. 400 Lynnwood, WA 98036 T (206) 667 0555

COST CONSULTANT Rider Levett Bucknall (RLB) Brewery Block 2 1120 NW Couch Street, Suite 730 Portland, OR 97209 T (503) 226 2730





REVISIONS DATE

Vancouver School District
FRANKLIN ELEMENTARY SCHOOL **ADDITION** 

PERSPECTIVE VIEWS

CONSTRUCTION OBSERVATION BY THE STRUCTURAL ENGINEER IS FOR GENERAL CONFORMANCE WITH DESIGN ASPECTS ONLY AND IS NOT INTENDED IN ANY WAY TO REVIEW THE CONTRACTOR'S CONSTRUCTION PROCEDURES.

ALL METHODS, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE 2015 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED AND ADOPTED BY THE LOCAL BUILDING OFFICIAL OR APPLICABLE JURISDICTION.

CONTRACT DRAWINGS / DIMENSIONS

STRUCTURES DURING CONSTRUCTION."

ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. CONSULTANT DRAWINGS BY OTHER DISCIPLINES ARE SUPPLEMENTARY TO ARCHITECTURAL DRAWINGS. REPORT DIMENSIONAL OMISSIONS OR DISCREPANCIES BETWEEN ARCHITECTURAL DRAWINGS AND STRUCTURAL, MECHANICAL, ELECTRICAL OR CIVIL DRAWINGS TO ARCHITECT PRIOR TO PROCEEDING WITH WORK.

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS. PRIMARY STRUCTURAL ELEMENTS ARE DIMENSIONED ON STRUCTURAL PLANS AND DETAILS AND OVERALL LAYOUT OF STRUCTURAL PORTION OF WORK. SOME SECONDARY ELEMENTS ARE NOT DIMENSIONED, SUCH AS WALL CONFIGURATIONS, INCLUDING EXACT DOOR AND WINDOW LOCATIONS, ALCOVES, SLAB SLOPES AND DEPRESSIONS CURBS, ETC. VERTICAL DIMENSIONAL CONTROL IS DEFINED BY ARCHITECTURAL WALL SECTIONS AND BUILDING SECTIONS. STRUCTURAL DETAILS SHOW DIMENSIONAL RELATIONSHIPS TO CONTROL DIMENSIONS DEFINED BY ARCHITECTURAL DRAWINGS. DETAILING AND SHOP DRAWING PRODUCTION FOR STRUCTURAL ELEMENTS WILL REQUIRE DIMENSIONAL INFORMATION CONTAINED IN **BOTH** ARCHITECTURAL AND STRUCTURAL DRAWINGS.

#### **DESIGN CRITERIA**

#### VERTICAL LOADS

AREA	DESIGN DEAD LOAD	LIVE LOAD (2)	ADDITIONAL LOAD	CONCENTRATED LOADS	
ROOF	15 PSF	27.5 PSF (1)	-	300#	
CLASSROOM	ACTUAL	40 PSF	-	1,000#	
MECHANICAL ROOM	15 PSF	40 PSF	+EQUIPMENT	-	
CORRIDORS (1ST FLOOR)	ACTUAL	100 PSF (3)	_	2,000#	

(1) SNOW LOAD PER BUILDING CODE IS GREATER THAN ROOF LIVE LOAD AND THUS CONTROLS DESIGN. (2) LIVE LOADS EXCEPT SNOW LOADS ARE REDUCED PER IBC SECTION 1607.10.

(3) LIVE LOAD REDUCTION NOT PERMITTED EXCEPT AS NOTED IN IBC SECTION 1607.10.

SNOW: (MINIMUM ROOF SNOW LOAD = Is x 25 PSF = 27.5 PSF)

Pg = 25 PSF = GROUND SNOW LOAD Pf = 0.7CeCt[sPg = FLAT ROOF SNOW LOAD

Ps = CsPf = SLOPED ROOF SNOW LOAD

Is = 1.1, Ce = 1.0, Ct = 1.0, Cs = 1.0

LATERAL FORCES

LATERAL FORCES ARE TRANSMITTED BY DIAPHRAGM ACTION OF ROOF AND FLOORS TO SHEAR WALLS. LOADS ARE THEN TRANSFERRED TO FOUNDATION BY SHEAR WALL ACTION WHERE ULTIMATE DISPLACEMENT IS RESISTED BY PASSIVE PRESSURE OF EARTH AND/OR SLIDING FRICTION. OVERTURNING IS RESISTED BY DEAD LOAD OF THE STRUCTURE.

WIND:

THE BUILDING MEETS THE CRITERIA TO USE THE "METHOD 2 - SIMPLIFIED ENVELOPE PROCEDURE" PER ASCE 7-10.

- EXPOSURE CATEGORY = B

50 SF

100 SF

500 SF

- BASIC WIND SPEED, (3 SEC. GUST), V<sub>ULT</sub> = 145 MPH; V<sub>ASD</sub> = 115 MPH
- RISK CATEGORY PER TABLE 1.5-1 = III - TOPOGRAPHIC FACTOR K<sub>ZT</sub> = 1.0
- INTERNAL PRESSURE COEFFICIENT (ENCLOSED) = ± 0.18
- COMPONENTS AND CLADDING LOADS, SEE THE FOLLOWING TABLES:

		RO	OF SURFACES 1					
	POSITIVE PRESSURES (PSF)			NEGATIVE PRESSURES (PSF)				
EFFECTIVE WIND AREA		ZONE <sup>2</sup>						
	1	2	3	1	2	3		
10 SF	16.0	16.0	16.0	-21.8	-37.9	-56.0		
20 SF	16.0	16.0	16.0	-21.2	-34.9	-52.4		
50 SF	16.0	16.0	16.0	-20.4	-30.9	-47.6		
100 SF	16.0	16.0	16.0	-19.8	-27.8	-44.0		
		WALL SURFACE	S AND ROOF OV	ERHANGS <sup>1</sup>				
	POSITIVE PR	ESSURE (PSF)	NEGATIVE PR	ESSURE (PSF)	ROOF OVER	HANGS (PSF)		
EFFECTIVE WIND AREA	ZONE <sup>2</sup>							
· <b></b> . ·	4	5	4	5	2	3		
10 SF	23.8	23.8	-25.8	-31.9	-44.4	-74.6		
20 SF	22.7	22.7	-24.7	-29.7	-44.4	-67.3		

-23.3

-22.2

-26.9

-24.7

-19.8

-44.4

-44.4

-57.7

-50.4

VALUES SHOWN IN TABLE ARE GROSS ULTIMATE WIND PRESSURES.
 ZONES ARE AS DEFINED BY FIGURE 30.5-1 IN ASCE 7-10.

21.3

20.2

17.7

21.3

20.2

17.7

SEISMIC: (ASCE 7-10) V = CsW

WHERE  $Cs = \frac{S_{DS}}{(\frac{R}{Ie})}$ ; WITH  $Cs \text{ MINIMUM} = 0.044 \text{ S}_{DS} I_E \ge 0.01$ OR  $Cs \text{ MINIMUM} = \frac{0.5S_1}{\frac{R}{Ie}} \text{ FOR } S_1 > 0.6g$   $Cs \text{ MAXIMUM} = \frac{S_{D1}}{T(\frac{R}{Ie})} \text{ FOR } T \le T_L$ OR  $Cs \text{ MAXIMUM} = \frac{S_{D1}T_L}{T^2(\frac{R}{L})} \text{ FOR } T \ge T_L$ 

SEISMIC IMPORTANCE FACTOR, Ie = 1.25 RISK CATEGORY OF BUILDING PER TABLE 1.5-1 = III SPECTRAL RESPONSE ACCELERATIONS Ss =  $0.923 \& S_1 = 0.403$ SITE CLASS PER TABLE 20.3-1 = D

DESIGN SPECTRAL RESPONSE ACCELERATIONS S<sub>DS</sub> = 0.696 & S<sub>D1</sub> = 0.429

SEISMIC DESIGN CATEGORY = D

W = EFFECTIVE SEISMIC WEIGHT OF BUILDING: WEST ADDITION = 122 K, NORTH ADDITION = 153 K

W = EFFECTIVE SEISMIC WEIGHT OF BUILDING: WEST ADDITION = 122 K, NORTH ADDITION = 153 K ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE PROCEDURE RESPONSE MODIFICATION FACTOR PER TABLE 12.2-1, R = 6.5

Cs = 0.134 DESIGN BASE SHEAR V: WEST ADDITION = 17 K, NORTH ADDITION = 21 K PIPES, DUCTS AND MECHANICAL EQUIPMENT SUPPORTED OR BRACED FROM STRUCTURE. CONFORM TO SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION, INC. PUBLICATION "SEISMIC RESTRAINT MANUAL: GUIDELINES FOR MECHANICAL SYSTEMS". SPRINKLER LINE ATTACHMENTS SHALL CONFORM TO NFPA PAMPHLET 13.

FOUNDATION DESIGN CRITERIA (PER GEODESIGN INC. DATED FEBRUARY 5, 2019).

\*1/2 INCREASE ALLOWED FOR SEISMIC OR WIND LOADING

SOIL BEARING PRESSURE: 3000 PSF\*

ACTIVE PRESSURE - UNRESTRAINED: 35 PCF +7.5H<sup>2</sup> SEISMIC SURCHARGE AT 0.6H FROM THE BASE OF THE WALL PASSIVE RESISTANCE: 300 PCF (INCLUDES F.O.S. ≥ 1.5) COEFFICIENT OF FRICTION: .35 (INCLUDES F.O.S. ≥ 1.5)

ALL FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED EARTH OR "STRUCTURAL BACKFILL". NATIVE EARTH BEARING SHALL BE SURFACE COMPACTED. AREAS OVER-EXCAVATED SHALL BE BACKFILLED WITH LEAN CONCRETE (F'c= 2000 PSI) OR "STRUCTURAL BACKFILL". AREAS DESIGNATED "STRUCTURAL BACKFILL" SHALL BE FILLED WITH APPROVED WELL-GRADED BANKRUN MATERIAL. MAXIMUM SIZE OF ROCK 4". FROZEN SOIL, ORGANIC MATERIAL AND DELETERIOUS MATTER NOT ALLOWED. COMPACT TO AT LEAST 95% OF ITS MAXIMUM DENSITY AS DETERMINED BY ASTM D1557. CONTRACTOR SHALL EXERCISE EXTREME CARE DURING EXCAVATION TO AVOID DAMAGE TO BURIED LINES, TANKS, AND OTHER CONCEALED ITEMS. UPON DISCOVERY, DO NOT PROCEED WITH WORK UNTIL RECEIVING WRITTEN INSTRUCTIONS FROM ARCHITECT. A COMPETENT REPRESENTATIVE OF THE OWNER SHALL INSPECT ALL FOOTING EXCAVATIONS FOR SUITABILITY OF BEARING SURFACES PRIOR TO PLACEMENT OF REINFORCING STEEL. PROVIDE DRAINAGE AND DEWATERING AROUND ALL WORK TO AVOID

WATER-SOFTENED FOOTINGS.
FREE DRAINING BACKFILL MATERIAL FOR RETAINING & BASEMENT WALLS

A CLEAN, FREE DRAINING, WELL GRADED GRANULAR MATERIAL CONFORMING TO ASTM D2487 GW OR SW WHOSE MAXIMUM PARTICLE SIZE DOES NOT EXCEED 3/4" AND WHOSE FINES CONTENT (MATERIAL PASSING THE NO. 200 SIEVE) DOES NOT EXCEED 5%.

WITH A MAXIMUM DUST RATIO

| With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | With a maximum dust ratio | W

#### **CONCRETE**

**CAST-IN-PLACE CONCRETE** 

MIX DESIGNS: THE CONTRACTOR SHALL DESIGN CONCRETE MIXES THAT MEET OR EXCEED THE REQUIREMENTS OF THE CONCRETE MIX TABLE. THE MIX DESIGNS SHALL FACILITATE ANTICIPATED PLACEMENT METHODS, WEATHER, REBAR CONGESTION, ARCHITECTURAL FINISHES, CONSTRUCTION SEQUENCING, STRUCTURAL DETAILS, AND ALL OTHER FACTORS REQUIRED TO PROVIDE A STRUCTURALLY SOUND, AESTHETICALLY ACCEPTABLE FINISHED PRODUCT. WATER REDUCING ADMIXTURES WILL LIKELY BE REQUIRED TO MEET THESE REQUIREMENTS. CONCRETE MIX DESIGNS SHALL CLEARLY INDICATE THE TARGET SLUMP. SLUMP TOLERANCE SHALL BE ± 1-1/2 INCHES.

AGGREGATE: COARSE AND FINE AGGREGATE SHALL CONFORM TO ASTM C33

<u>CEMENT</u>: CEMENT SHALL CONFORM TO ASTM C150, TYPE II PORTLAND CEMENT, UNLESS NOTED OTHERWISE.

FLYASH: SHALL CONFORM TO ASTM C618 CLASS C OR F, MAXIMUM LOSS OF IGNITION SHALL BE 1.0%.

SLAG: GROUND GRANULATED BLAST-FURNACE (GGBF) SLAG SHALL CONFORM TO ASTM C989 GRADE 100 OR 120.

ALTERNATE MIX DESIGNS: VARIATIONS TO THE MIX DESIGN PROPORTIONS MAY BE ACCEPTED IF SUBSTANTIATED IN ACCORDANCE WITH ACI 318, CHAPTER 19. PROVIDE SUBMITTALS A MINIMUM OF TWO WEEKS PRIOR TO BID FOR DETERMINATION OF ACCEPTABILITY.

ADMIXTURES: ADMIXTURES SHALL BE BY MASTER BUILDERS, W.R. GRACE, OR PRE-APPROVED EQUAL. ALL MANUFACTURER'S RECOMMENDATIONS SHALL BE FOLLOWED.

WATER: SHALL BE CLEAN AND POTABLE.

MAXIMUM CHLORIDE CONTENT: THE MAXIMUM WATER SOLUBLE CHLORIDE CONTENT SHALL NOT EXCEED 0.15% BY WEIGHT OF CEMENTITIOUS MATERIAL UNLESS NOTED OTHERWISE.

CONCRETE EXPOSED TO WEATHER: PROVIDE 5.0% TOTAL AIR CONTENT FOR ALL CONCRETE EXPOSED TO WEATHER. TOTAL AIR CONTENT IS THE SUM OF ENTRAINED AIR PROVIDED BY ADMIXTURES AND NATURALLY OCCURRING ENTRAPPED AIR. AIR CONTENT SHALL BE TESTED PRIOR TO BEING PLACED IN THE PUMP HOPPER OR BUCKET; IT IS NOT REQUIRED TO BE TESTED AT THE DISCHARGE END OF THE PUMP HOSE. THE TOLERANCE ON ENTRAPPED AIR SHALL BE +2.0% AND -1.5% WITH THE AVERAGE OF ALL TESTS NOT LESS THAN THE SPECIFIED

TOTAL CEMENTITIOUS MATERIAL: THE SUM OF ALL CEMENT PLUS FLYASH AND SLAG. AT THE CONTRACTORS OPTION FLYASH OR SLAG MAY BE SUBSTITUTED FOR CEMENT BUT SHALL NOT EXCEED 25% BY WEIGHT OF TOTAL CEMENTITIOUS MATERIAL. IN NO CASE SHALL THE AMOUNT OF FLYASH OR SLAG BE LESS THAN REQUIRED BY THE CONCRETE MIX DESIGN TABLE. FOOTING MIXES SHALL CONTAIN NOT LESS THAN 5 SACKS OF CEMENTITIOUS MATERIAL PER CUBIC YARD, ALL OTHER MIXES SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF CEMENTITIOUS MATERIAL PER CUBIC YARD, UNLESS NOTED OTHERWISE.

ITEM	DESIGN fc (PSI) (AT 28 DAYS U.N.O.)	MAX. W/C RATIO	MIN. FLYASH OR SLAG (PCY)	AGGREGATE GRADING ASTM AASHTO	NOTES
SLAB ON GRADE - EXPOSED TO WEATHER	5000	0.40	100	57 OR 67	1
SLABS ON GRADE - UNO	4000	0.45	100	57 OR 67	1
FOUNDATIONS - UNO	3000	0.50		57 OR 67	
STEM WALLS AND OTHER WALLS EXPOSED TO EARTH OR WEATHER	4500	0.45	100	57 OR 67	
STEM WALLS AND OTHER WALLS - UNO	4000	0.50	100	57 OR 67	
ALL OTHER CONCRETE	4000	0.50		57 OR 67	

## CONCRETE MIX NOTES

1. FIBROUS CONCRETE REINFORCEMENT SHALL BE "FIBERMESH" MANUFACTURED BY PROPEX CONCRETE SYSTEMS OR PRE-APPROVED EQUAL AND SHALL CONFORM TO ASTM C1116 TYPE III 4.1.3, PERFORMANCE LEVEL 1, AND SHALL BE 100 PERCENT VIRGIN POLYPROPYLENE, FIBRILLATED FIBERS CONTAINING NO REPROCESSED OLEFIN MATERIALS AND SPECIFICALLY MANUFACTURED FOR USE AS CONCRETE SECONDARY REINFORCEMENT. DOSAGE SHALL FOLLOW MANUFACTURER'S RECOMMENDATION BUT NOT LESS THAN 1.5 LB/CU. YD.

## CONCRETE PLACEMENT

PLACE CONCRETE FOLLOWING ALL APPLICABLE ACI RECOMMENDATIONS. CONCRETE SHALL BE PROPERLY CONSOLIDATED PER ACI 309 USING INTERIOR MECHANICAL VIBRATORS, DO NOT OVER-VIBRATE. CONCRETE SHALL BE POURED MONOLITHICALLY BETWEEN CONSTRUCTION OR EXPANSION JOINTS. IF CONCRETE IS PLACED BY THE PUMP METHOD, HORSES SHALL BE PROVIDED TO SUPPORT THE HOSE, THE HOSE SHALL NOT BE ALLOWED TO RIDE ON THE REINFORCING. WEATHER FORECASTS SHALL BE MONITORED AND ACI RECOMMENDATIONS FOR HOT AND COLD WEATHER CONCRETING SHALL BE FOLLOWED AS REQUIRED. CONCRETE SHALL NOT FREE FALL MORE THAN 5 FEET DURING PLACEMENT WITHOUT WRITTEN APPROVAL OF STRUCTURAL ENGINEER.

FLOATING & FINISHING OPERATIONS

WATER SHALL NOT BE ADDED TO THE CONCRETE SURFACE DURING FLOATING & FINISHING OPERATIONS. PRE-APPROVED EVAPORATION RETARDER SPECIFICALLY DESIGNED FOR FLOATING & FINISHING OPERATIONS ARE ACCEPTABLE.

## FORMED SURFACES:

FORMWORK CLASS OF SURFACE PER ACI 347 TABLE 3.1	
ITEM	CLASS OF FINISH
ALL SURFACES EXPOSED TO PUBLIC VIEW, U.N.O.	Α
ALL SURFACES RECEIVING A COURSE TEXTURED COATING SUCH AS PLASTER OR STUCCO, UNLESS NOTED OTHERWISE	В
ALL OTHER SURFACES, UNLESS NOTED OTHERWISE	С

#### FORMWORK STRIPPING:

STEM WALLS: STEM WALLS NOT SUPPORTING FRAMING WEIGHT MAY BE STRIPPED AS SOON AS FORMS CAN BE REMOVED WITHOUT DAMAGING THE CONCRETE AND THE CONCRETE HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 500 PSI.

#### **COLD WEATHER PLACEMENT:**

- 1. COLD WEATHER IS DEFINED BY ACI 306 AS "A PERIOD WHEN FOR MORE THAN 3 SUCCESSIVE DAYS THE MEAN DAILY TEMPERATURE DROPS BELOW 40° F."
- 2. NO CONCRETE SHALL BE PLACED ON FROZEN OR PARTIALLY FROZEN GROUND. THAWING THE GROUND WITH HEATERS IS PERMISSIBLE
- WITH HEATERS IS PERMISSIBLE.

  3. CONCRETE MIX TEMPERATURES SHALL BE AS SHOWN BELOW. HEATING OF WATER AND/OR AGGREGATES
- MAY BE REQUIRED TO ATTAIN THESE TEMPERATURES.

  4. THE CONCRETE MAY REQUIRE PROTECTION FOR 4-7 DAYS AFTER POURING. IF TEMPERATURES REMAIN BELOW FREEZING, INSULATING BLANKET COVERAGE IS REQUIRED. IF TEMPERATURES ARE SLIGHTLY

BELOW FREEZING (30° F MIN.) AT NIGHT AND ABOVE FREEZING DURING THE DAY, KRAFT PAPER WITH

5. NO ADDITIVES CONTAINING CHLORIDES SHALL BE USED. USE "POZZUTEC 20+" BY MASTER BUILDERS OR "POLARSET" BY W.R. GRACE OR PRE-APPROVED EQUAL.

COMPLETE COVERAGE MAY BE USED IN LIEU OF INSULATED BLANKETS.

CONDITION OF PLACEMENT AND CURING		WALLS & SLABS	FOOTINGS
MIN. TEMP. FRESH CONCRETE AS MIXED FOR WEATHER INDICATED, DEGREES F.	ABOVE 30° F. 0° TO 30° F. BELOW 0° F.	60° 65° 70°	55° 60° 65°
MIN. TEMP. FRESH CONCRETE AS PLACED AN	55°	50°	
MAX. ALLOWABLE GRADUAL DROP IN TEMP. T HOURS AFTER END OF PROTECTION, DEGREE	50°	40°	

HOT OR WINDY WEATHER PLACEMENT

HOT WEATHER IS DEFINED BY ACI 305 AS "ANY COMBINATION OF HIGH AIR TEMPERATURE, LOW RELATIVE HUMIDITY, AND WIND VELOCITY, TENDING TO IMPAIR THE QUALITY OF FRESH HARDENED CONCRETE. ACI 305 FIGURE 2.1.5 SHALL BE USED BY THE CONTRACTOR TO ESTIMATE THE RATE OF EVAPORATION. WHEN THE ESTIMATED RATE OF EVAPORATION IS GREATER THAN 0.2 PSF/HOUR THE PLACEMENT SHALL BE CONSIDERED A HOT WEATHER PLACEMENT. PRECAUTIONS AGAINST PLASTIC SHRINKAGE CRACKING ARE NECESSARY. PRECAUTIONS TAKEN BY THE CONTRACTOR VARY DEPENDING UPON THE FACTORS ASSOCIATED WITH WATER EVAPORATION AND INCLUDE BUT ARE NOT LIMITED TO:

1. LIMITING CONCRETE TEMPERATURE TO 100°F AT TIME OF PLACEMENT.

2. APPLICATION OF AN EVAPORATION RETARDER.

3. USE OF FOG SPRAY.

4. REDUCTION OF POUR SIZE.

5. PLACING CONCRETE AT NIGHT.

#### **CONTROL AND CONSTRUCTION JOINTS**

CONSTRUCTION JOINTS SHALL MEET THE REQUIREMENTS OF ACI 301 SECTIONS 2.2.2.5 AND 5.3.2.6. SPECIAL BONDING METHODS PER SECTION 5.3.2.6 SHALL BE SATISFIED BY ITEM 3 BELOW UNLESS OTHERWISE DETAILED ON THE STRUCTURAL DRAWINGS. WHERE CONSTRUCTION JOINTS ARE NOT SHOWN ON PLAN OR ADDITIONAL CONSTRUCTION JOINTS ARE REQUIRED SUBMIT PROPOSED JOINTING FOR STRUCTURAL ENGINEERS APPROVAL. PROVIDE CONSTRUCTION JOINTS AS INDICATED BELOW UNLESS NOTED OTHERWISE ON THE PLANS:

- 1. SLABS ON GRADE: PROVIDE CONSTRUCTION AND/OR CONTROL JOINTS AT 16 FEET O.C. MAXIMUM FOR UNEXPOSED SLABS ON GRADE AND 12 FEET O.C. FOR EXPOSED SLABS ON GRADE. COORDINATE JOINTS WITH ARCHITECTURAL DRAWINGS.
- 2. BONDING AGENT: WHERE BONDING AGENT IS SPECIFICALLY CALLED OUT ON THE STRUCTURAL DRAWINGS USE "WELD CRETE" BY LARSON PRODUCTS CORPORATION OR PRE-APPROVED EQUAL. FOLLOW ALL MANUFACTURERS RECOMMENDATIONS.
- ATTACHMENT OF NEW CONCRETE TO EXISTING: WHERE SHOWN, ROUGHEN CONCRETE TO A MINIMUM AMPLITUDE OF 1/4" USING IMPACT HAMMER. REMOVE ALL LOOSE OR DAMAGED CONCRETE, THOROUGHLY FLUSH ALL SURFACES WITH POTABLE WATER, AIR BLAST WITH OIL FREE COMPRESSED AIR TO REMOVE ALL WATER

# WATER. EMBEDDED ITEMS

- 1. NO ALUMINUM ITEMS SHALL BE EMBEDDED IN ANY CONCRETE
- 2. ALL EMBED PLATES SHALL BE SECURELY FASTENED IN PLACE
- 3. ALL EMBEDDED STEEL ITEMS EXPOSED TO EARTH SHALL BE GALVANIZED.
- 4. ALL EMBEDDED STEEL ITEMS EXPOSED TO WEATHER SHALL BE PAINTED UNLESS NOTED AS GALVANIZED. SEE DRAWINGS AND SPECIFICATIONS FOR PAINT, PRIMER, AND GALVANIZING REQUIREMENTS.

## CONCRETE CURING AND SEALING

CURING PROCEDURES SHALL COMMENCE IMMEDIATELY AFTER FINISHING CONCRETE TO MAINTAIN CONCRETE IN A MOIST CONDITION. VERIFY CURING AND/OR SEALING PRODUCTS ARE COMPATIBLE WITH FLOOR COVERINGS SHOWN ON THE ARCHITECTURAL DRAWINGS. FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS. SLABS ARE DEFINED AS SLABS ON GRADE, CONCRETE ON METAL DECK, ELEVATED POST-TENSIONED OR MILD REINFORCED DECKS. AND TOPPING SLABS.

ITEM	CONCRETE CURING NOTES
SLABS EXPOSED TO EARTH OR WEATHER	1, (3 OR 4 OR 5), 6
ALL OTHER SLABS	1, (3 OR 4 OR 5)
FORMED SURFACES EXCLUDING FOUNDATIONS	2
ALL OTHER CONCRETE	NONE

## CONCRETE CURING NOTES:

- WHEN THE ESTIMATED EVAPORATION RATE IS GREATER THAN 0.2 PSF/HOUR PROVIDE A SPRAY APPLIED EVAPORATION RETARDER IMMEDIATELY AFTER CONCRETE PLACEMENT. THE EVAPORATION RATE MAY BE CALCULATED PER ACI 305 FIGURE 2.1.5.
- 2. APPLY A LIQUID MEMBRANE FORMING CURING COMPOUND, CONFORMING TO ASTM C309 TYPE 1 CLASS B SPECIFICATIONS, PER MANUFACTURER'S RECOMMENDATIONS TO ALL FORMED SURFACES IMMEDIATELY AFTER FINAL FORM REMOVAL. NOT REQUIRED IF FORMWORK REMAINS IN PLACE FOR MORE THAN 7 DAYS.
- B. PROVIDE PRE-APPROVED CONTINUOUS WET CURE METHOD FOR A MINIMUM OF 14 DAYS.
- I. APPLY A LIQUID MEMBRANE FORMING CURING COMPOUND, CONFORMING TO ASTM C309 TYPE 1 CLASS B SPECIFICATIONS OR ASTM C1315 TYPE 1 CLASS A SPECIFICATIONS, PER MANUFACTURER'S RECOMMENDATIONS IMMEDIATELY AFTER FINAL FINISHING. CURING COMPOUND SHALL BE COMPATIBLE WITH ARCHITECTURAL FLOOR COVERINGS AND SEALERS.
- PROVIDE 'ULTRACURE MAX' MOISTURE RETAINING COVER BY MCTECH GROUP, OR APPROVED EQUAL, FOR A MINIMUM OF 14 DAYS.
- 6. APPLY A SILANE SEALER WITH MINIMUM SOLIDS CONTENT OF 40% PER MANUFACTURER'S RECOMMENDATIONS.

## GROUT

NON-SHRINK GROUT: MASTER BUILDERS "MASTERFLOW 928" OR PRE-APPROVED EQUAL. GROUT SHALL CONFORM TO CRD-C621 AND ASTM C1107 WHEN TESTED AT A FLUID CONSISTENCY PER CRD-C611-85 FOR 30 MINUTES. GROUT MAY BE PLACED FROM A 25 SECOND FLOW TO A STIFF PACKING CONSISTENCY. FILL OR PACK ENTIRE SPACE UNDER PLATES OR SHAPES. FOLLOW MANUFACTURER'S RECOMMENDATIONS FOR PREPARATION, INSTALLATION. AND CURING.

# REINFORCING STEEL

REINFORCING STEEL SHALL CONFORM TO:

ASTM A615, GRADE 60 TYPICAL UNLESS NOTED OTHERWISE.

DETAIL FABRICATE AND PLACE PER ACI 315 AND ACI 318.

<u>R</u> l	EINFORCING SPLICE	AND DEVELOPMENT LE	ENGTH SCHEDULE, Fy=	60 KSI (UNLESS NOTED	OTHERWISE)	
BAR	MINIMUM LAP SPL	ICE LENGTHS ("Ls")	MINIMUM DEVELOP	MINIMUM EMBEDMENT		
SIZE	TOP BARS (1)	OTHER BARS	TOP BARS (1)	OTHER BARS	LENGTH FOR STANDARD END HOOKS ("Ldh")	
#3	2'-0"	1'-6"	1'-6"	1'-3"	0'-7"	
#4	2'-8"	2'-0"	2'-0"	1'-7"	0'-9"	
#5	3'-4"	2'-7"	2'-7"	2'-0"	1'-0"	
#6	4'-0"	3'-1"	3'-1"	2'-4"	1'-2"	

SPLICE TABLE NOTES:

1. "TOP BARS" ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THEM.

MECHANICAL COUPLERS: "LENTON" BY ERICO, "CADWELD" BY ERICO, "BAR-LOCK" BY DAYTON SUPERIOR L-SERIES, OR PRE-APPROVED EQUAL. COUPLERS SHALL BE TYPE 2 PER ACI 318 SECTION 18.2.7.1.

FORM SAVERS: "LENTON" BY ERICO THREADED FORM SAVERS TYPE FS OR APPROVED EQUAL.

#### REINFORCING STEEL COVER

PROVIDE CONCRETE COVER OVER REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST EARTH ----- 3"
EXPOSED TO WEATHER OR EARTH ----- 2"
TIES ON BEAMS AND COLUMNS ----- 1-1/2"
WALLS AND SLABS NOT EXPOSED TO WEATHER---- 3/4"

CONCRETE INSERTS: THREADED DOWEL BAR SUBSTITUTIONS SHALL BE MANUFACTURED BY RICHMOND SCREW ANCHOR CO., INC., OR PRE-APPROVED EQUAL AND SHALL BE CAPABLE OF DEVELOPING THE FULL TENSILE CAPACITY OF THE BAR.

#### POST-INSTALLED ANCHORS

POST-INSTALLED ANCHORS: SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH REBAR. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS. INSTALLER SHALL BE QUALIFIED AND TRAINED BY THE MANUFACTURER. HOLES SHALL BE HAMMER DRILLED ONLY (ROTARY DRILLED ONLY AT UNREINFORCED MASONRY - NO HAMMER TOOLS).

SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW, SHALL BE SUBMITTED FOR APPROVAL A MINIMUM OF 2 WEEKS PRIOR TO BID, ALONG WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER (LICENSED IN THE STATE IN WHICH THE PROJECT OCCURS) DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

CONCRETE ANCHORS:
- ADHESIVE ANCHORS: HILTI HIT-HY 200 (ICC-ESR-3187), HILTI HIT-RE 500 VE (ICC-ESR-3814), DEWALT PURE

\*CONCRETE SHALL BE IN THE TEMPERATURE RANGE AS REQUIRED BY THE CONCRETE

MANUFACTURER.
\*HOLE SHALL BY HAMMER-DRILLED ONLY.

\*HOLE SHALL BE DRY AT TIME OF INSTALLATION.

\*INSTALLER OF HORIZONTAL OR UPWARDLY INCLINED (ANY POSITION EXCEPT DIRECTLY DOWNWARD) ANCHORS SHALL ALSO BE CERTIFIED BY THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM.

- EXPANSION ANCHORS: KWIKBOLT TZ (ICC ESR-1917) BY HILTI, INC. OR PRE-APPROVED EQUAL

- SCREW ANCHORS: KWIK HUS-EZ (ICC ESR-3027) BY HILTI, INC. OR PRE-APPROVED EQUAL.

SHEET NUMBER	SHEET DESCRIPTION		
S0.00	PERSPECTIVE VIEWS		
S0.01	GENERAL NOTES		
S0.02	GENERAL NOTES		
S0.03	GENERAL NOTES		
S2.01	FOUNDATION AND GRADE LEVEL FRAMING PLANS		
S2.02	ATTIC FRAMING PLANS		
S2.03	ROOF FRAMING PLANS		
S3.01	FOUNDATION DETAILS		
S3.02	FOUNDATION DETAILS		
S4.01	WOOD FRAMING DETAILS		
S4.02	WOOD FRAMING DETAILS		
S4.03	WOOD FRAMING DETAILS		
S4.04	WOOD FRAMING DETAILS		
S4.05	WOOD FRAMING DETAILS		
S4.06	WOOD FRAMING DETAILS		
Grand total: 15			

**bassetti** architects

Jurisdiction Stamp Area

71 Columbia Street, Suite 500 Seattle, Washington 98104 T (206) 340 9500 F (206) 340 9519

CIVIL ENGINEER AND LANDSCAPE MacKay Sposito 1325 SE Tech Center Drive, Suite 140 Vancouver, WA 98683 T (360) 695 3411

STRUCTURAL ENGINEER
PCS Structural Solutions
One Main Place
101 SW Main Street, Suite 280
Portland, OR 97204
T (503) 232 3746

MECHANICAL ENGINEER
BCE Engineers
6021 12th St E, Suite 200
Fife, WA 98424
T (253) 922-0446

ELECTRICAL ENGINEER BCE Engineers 6021 12th St E, Suite 200 Fife, WA 98424 T (253) 922-0446

ACOUSTIC

4100 194th St., SW, Ste. 400 Lynnwood, WA 98036 T (206) 667 0555 COST CONSULTANT Rider Levett Bucknall (RLB) Brewery Block 2

1120 NW Couch Street, Suite 730

Portland, OR 97209

T (503) 226 2730

ONE INCH





REVISIONS DATE

Vancouver School District

FRANKLIN
ELEMENTARY
SCHOOL
ADDITION

JOB NO: 18790

ISSUE DATE: 08/19/2019

Stamp Area

5206 NW Franklin St,

Vancouver, WA 98663

GENERAL NOTES

S0.01

#### **MASONRY**

MASONRY VENEER: SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 14 OF THE IBC, AND THE PROJECT SPECIFICATIONS.

MORTAR: SHALL BE TYPE S PER IBC. CONFORM TO ASTM C270. MINIMUM COMPRESSIVE STRENGTH = 1800 PSI.

REQUIREMENTS FOR ALL-WEATHER MASONRY CONSTRUCTION: HOT AND COLD WEATHER CONSTRUCTION REQUIREMENTS SHALL BE IN ACCORDANCE WITH TMS 602-13 / ACI 530.1-13 / ASCE 6-13 "SPECIFICATION FOR MASONRY STRUCTURES", ARTICLES 1.8C AND 1.8D.

ANCHORED VENEER (MASONRY AND STONE UNITS): ALL VENEER ANCHORAGE ATTACHMENTS SHALL CONFORM TO IBC SECTION 1405.6 AND ACI 530 SECTIONS 12.1 AND 12.2 FOR THE APPLICABLE SEISMIC DESIGN CATEGORY.

ANCHOR TIES AND JOINT REINFORCEMENT SHALL BE STAINLESS STEEL AND SHALL BE MANUFACTURED BY WIRE-BOND OR HOHMANN & BARNARD OR PRE-APPROVED EQUAL. ANCHOR TIES SHALL BE SPACED 16"O.C. EACH WAY MAXIMUM, AND SHALL HAVE A LIP OR HOOK ON THE EXTENDED LEG THAT WILL ENGAGE OR ENCLOSE A HORIZONTAL JOINT REINFORCEMENT WIRE OF NO. 9 GAUGE OR EQUIVALENT. THE JOINT REINFORCEMENT SHALL BE CONTINUOUS WITH BUTT SPLICES BETWEEN TIES PERMITTED.

#### ANCHORAGE OF VENEER TO BACKING SHALL BE AS FOLLOWS:

BACKING	VENEER TIE	ATTACHMENT TO BACKING
WOOD STUDS	WIRE-BOND RJ-711, HOHMANN & BARNARD HB-213 S.I.S. OR HOHMANN & BARNARD 2-SEAL THERMAL WING NUT	STAINLESS STEEL SCREWS BY MANUFACTURER
CONCRETE	WIRE-BOND RJ-711, HOHMANN & BARNARD HB-213 S.I.S. OR HOHMANN & BARNARD 2-SEAL THERMAL WING NUT	1/4" HILTI KWIK-CON II + SCREW W/ 1-3/4" EMBEDMENT
STRUCTURAL STEEL	WIRE-BOND TYPE I WELD ON ANCHOR W/ TRIANGULAR TIE & WIRE-BOND CLIP OR HOHMANN & BARNARD HB-359 WELD ON ANCHOR W/ VEE BYNA-TIE	WELD ON CLIP

<u>ANCHOR VENEER WITH CAVITY WIDTH GREATER THAN 4-1/2": WHERE THE CAVITY WIDTH BETWEEN THE</u> EXTERIOR FACE OF THE STRUCTURAL BACKING WALL AND THE INTERIOR FACE OF THE VENEER IS GREATER THAN 4-1/2", VENEER ANCHORAGE SYSTEMS SHALL BE DESIGNED PER THE REQUIREMENTS OF CURRENT CODE AND THE PROJECT SPECIFICATIONS. PROVIDE CALCULATIONS AND SHOP DRAWINGS INDICATING THE TYPE, SIZE, AND REQUIRED SPACING OF VENEER ANCHORS. CALCULATIONS AND SHOP DRAWINGS SHALL BE PRODUCED UNDER THE SUPERVISION OF AND SHALL BEAR THE STAMP OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF WASHINGTON.

#### STRUCTURAL STEEL

#### DETAILING, FABRICATION AND ERECTION

ALL WORKMANSHIP SHALL CONFORM TO THE AISC MANUAL OF STEEL CONSTRUCTION, 14TH EDITION, THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS JUNE 22, 2010, THE AISC CODE OF STANDARD PRACTICE, APRIL 14, 2010 AND THE AISC SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS, JUNE 22, 2010.

STEEL MEMBERS ARE EQUALLY SPACED BETWEEN COLUMNS AND/OR DIMENSION POINTS UNLESS NOTED OTHERWISE.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDES AND JOINT PREPARATIONS THAT INCLUDE BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDES, WELDING PROCEDURES. REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, WELD EXTENSION TABS, COPES, SURFACE ROUGHNESS VALUES AND TAPERS OF UNEQUAL PARTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLIANCE WITH ALL CURRENT OSHA REQUIREMENTS.

HOLES, COPES OR OTHER CUTS OR MODIFICATIONS OF THE STRUCTURAL STEEL MEMBERS SHALL NOT BE MADE IN THE FIELD WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER.

#### STEEL FABRICATORS

ALL STEEL FABRICATION SHALL BE PERFORMED BY A FABRICATOR CERTIFIED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION. THE FABRICATOR SHALL BE DESIGNATED AN AISC CERTIFIED PLANT, CATEGORY STD AT THE TIME OF BID AND SHALL MAINTAIN THIS CERTIFICATION FOR THE DURATION OF THE PROJECT.

NON-AISC CERTIFIED STEEL FABRICATORS SHALL HAVE FIVE YEARS MINIMUM EXPERIENCE ON SIMILAR PROJECTS OF EQUAL OR LARGER COMPLEXITY AND SCOPE. QUALIFICATIONS SHALL BE SUBMITTED TWO WEEKS PRIOR TO BID.

#### STEEL ERECTORS

ALL STEEL ERECTION SHALL BE PERFORMED BY AN ERECTOR CERTIFIED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION. THE ERECTOR SHALL BE DESIGNATED AN AISC CERTIFIED ERECTOR, CATEGORY CSE AT THE TIME OF BID AND SHALL MAINTAIN THIS CERTIFICATION FOR THE DURATION OF THE PROJECT.

NON-AISC CERTIFIED STEEL ERECTORS SHALL HAVE FIVE YEARS MINIMUM EXPERIENCE ON SIMILAR PROJECTS OF EQUAL OR LARGER COMPLEXITY AND SCOPE. QUALIFICATIONS SHALL BE SUBMITTED TWO WEEKS PRIOR TO BID.

## STEEL DETAILERS

ALL STEEL DETAILING SHALL BE PERFORMED BY A DETAILER WITH FIVE YEARS MINIMUM EXPERIENCE ON SIMILAR PROJECTS OF EQUAL OR LARGER COMPLEXITY AND SCOPE. QUALIFICATIONS SHALL BE SUBMITTED TWO WEEKS PRIOR TO BID.

## MATERIAL PROPERTIES

OTHER SHAPES AND PLATES: ASTM A36 (Fy = 36 KSI) TYP. U.N.O.; ASTM A572 (Fy = 50 KSI) WHERE INDICATED

HOLLOW STRUCTURAL SECTIONS: RECTANGULAR & SQUARE - ASTM A500 GRADE C (Fy = 50 KSI) ROUND - ASTM A500 GRADE C (Fy = 46 KSI)

## MACHINE BOLTS (M.B.): ASTM A307, GRADE A

ANCHOR BOLTS (A.B.): ASTM F1554, GRADE 36, UNLESS OTHERWISE NOTED, ASTM F1554, GRADE 105 WHERE INDICATED.

## **WELDING**

STRUCTURAL STEEL: WELD IN ACCORDANCE WITH "STRUCTURAL WELDING CODE" AWS D1.1

CERTIFICATION: ALL WELDING SHALL BE PERFORMED BY WABO/AWS CERTIFIED WELDERS. WELDERS SHALL BE PREQUALIFIED FOR EACH POSITION AND WELD TYPE WHICH THE WELDER WILL BE PERFORMING.

THE PROCESS CONSUMABLES FOR ALL WELD FILLER METAL INCLUDING TACK WELDS, ROOT PASS AND SUBSEQUENT PASSES DEPOSITED IN A JOINT SHALL BE COMPATIBLE.

ALL WELD FILLER METAL AND WELD PROCESS SHALL PROVIDE THE TENSILE STRENGTH CHARPY V-NOTCH **RATINGS AS FOLLOWS:** 

## **GRAVITY FRAME**

WELD TYPE	FILLER METAL TENSILE STRENGTH	CHARPY V-NOTCH (CVN) RATING
FILLET	70 KSI	
PARTIAL PENETRATION	70 KSI	

## WELDED CONNECTIONS INSPECTION:

ALL WELDING SHALL BE CHECKED BY VISUAL MEANS AND BY OTHER METHODS DEEMED NECESSARY BY THE WELDING INSPECTOR.

THE STANDARDS OF ACCEPTANCE FOR WELDS TESTED BY ULTRASONIC METHODS SHALL CONFORM TO AWS D1.1.

ALL WELDS FOUND TO BE DEFECTIVE SHALL BE REPAIRED AND REINSPECTED BY THE SAME METHODS ORIGINALLY USED. AND THIS REPAIR AND REINSPECTION SHALL BE PAID FOR BY THE CONTRACTOR

#### **GENERAL REQUIREMENTS**

BOLTED CONNECTIONS INSPECTION: CONNECTIONS MADE WITH BEARING TYPE BOLTS SHALL BE INSPECTED PER SECTION 9.1 AND CONNECTIONS MADE WITH SLIP-CRITICAL TYPE BOLTS (A325SC OR A490SC) SHALL BE INSPECTED PER SECTION 9.3 OF RCSC SPECIFICATION.

ADHESIVE ANCHOR RODS: ASTM F1554, GRADE 36 UNLESS NOTED OTHERWISE.

FINISH: STRUCTURAL STEEL SHALL BE PAINTED, UNLESS NOTED OTHERWISE, AND SHALL BE CLEAN OF LOOSE RUST, LOOSE MILL SCALE, OIL, GREASE AND OTHER FOREIGN SUBSTANCES AND SHALL MEET THE REQUIREMENTS OF SSPC-SP1. WHERE STRUCTURAL STEEL IS NOTED TO BE PAINTED, ALL AREAS COMPRISING THE FAYING SURFACES OF BOLTED CONNECTIONS MADE WITH SLIP-CRITICAL TYPE BOLTS (A325SC OR A490SC) SHALL COMPLY WITH THE REQUIREMENTS OF THE RCSC SPECIFICATION. WHERE STRUCTURAL STEEL IS NOTED TO BE GALVANIZED, IT SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123, A384, AND A385. ALL SURFACES WITHIN TWO INCHES OF ANY FIELD WELD LOCATION SHALL BE FREE OF MATERIALS THAT WOULD PREVENT PROPER WELDING OR PRODUCE OBJECTIONABLE FUMES. FIELD TOUCH-UP OF PRIMED, PAINTED, AND GALVANIZED SURFACES SHALL BE PERFORMED TO REPAIR COATING ABRASIONS, AS WELL AS TO PROTECT ALL AREAS AT CONNECTIONS.

#### CARPENTRY

NAILS: CONNECTION DESIGNS ARE BASED ON "COMMON WIRE" NAILS WITH THE FOLLOWING PROPERTIES:

PENNYWEIGHT	DIAMETER (INCHES)	LENGTH (INCHES)	TRACKER** EMBOSSED HEAD / COLOR
8d	0.131	2-1/2	3 / BLUE
10d	0.148	3	4 / WHITE
16d	0.162	3-1/2	6 / ORANGE
20d	0.192	4	-

FOR DIAPHRAGM OR SHEAR WALL NAILING THE FOLLOWING FASTENER TYPES MAY BE USED AT EQUIVALENT SPACING TO THAT SPECIFIED ON PLANS:

FASTENER TYPE	DIAMETER (INCHES)	LENGTH (INCHES)	EQUIVALENT SPACING (INCHES)			TRACKER** EMBOSSED HEAD / COLOR
8d COMMON WIRE	0.131	2-1/2	6	4	3	3/ BLUE
8d "DIPPED GALV. BOX" 8d "SHINY BOX" 12 GA. STAPLES 14 GA. STAPLES 15 GA STAPLES	0.131 0.113 0.1055 0.080 0.072	2-1/2 2-1/2 1-7/8* 1-1/2* 1-1/2*	6 4-1/2 6 6 5	4 3 5-1/2 4 3	3 2-1/2 4 3 2-1/2	E3 / NONE 1 / BLUE - - -
10d COMMON WIRE	0.148	3	6	4	3	4 / WHITE
10d "HOT DIPPED GALV. BOX" 10d "SHINY BOX"	0.148 0.128	3 3	6 4-1/2	4 3	3 2-1/4	F4 / NONE 3 / WHITE

\*BASED ON 15/32" PLYWOOD OR OSB.

\*\*REFERENCE TO EMBOSSED HEAD / COLOR CODED NAILS PER TRACKERS SYSTEM

<u>WOOD SHEATHING (STRUCTURAL)</u>: **SHEATHING ON ROOF SURFACES SHALL BE <u>PLYWOOD ONLY</u>. SHEATHING ON** FLOOR AND WALLS SHALL BE PLYWOOD OR ORIENTED STRAND BOARD (OSB). PLYWOOD SHEATHING SHALL BE 5-PLY MINIMUM WHERE INDICATED AS PERFORMANCE CATEGORY 3/4" OR THICKER. WOOD SHEATHING SHALL BE "STRUCTURAL I" CONFORMING TO PS1-09 AND/OR PS2-10. ALL PANELS SHALL BEAR THE STAMP OF AN APPROVED GRADING AGENCY. SPAN RATING SHALL BE PROVIDED AS FOLLOWS: ROOF FRAMING AT 32"O.C. (48/24): ROOF FRAMING AT 24"O.C. (32/16); WALLS (32/16); FLOORS (48/24) ALL WOOD SHEATHED WALLS SHALL BE BLOCKED AT ALL PANEL EDGES UNLESS NOTED OTHERWISE.

GLUE-LAMINATED MEMBERS: CONFORM TO ANSI/AITC A190.1. MEMBERS SHALL BE COMBINATION 24F-V4 DOUGLAS FIR (DF) FOR SIMPLE SPANS AND 24F-V8 DF FOR CANTILEVERED SPANS (Fb=2400 PSI, Fv=265 PSI, E= 1.8X10<sup>6</sup> PSI) AND DF COMBINATION 2 FOR COLUMNS.

ARCHITECTURAL APPEARANCE GRADE WHERE EXPOSED TO VIEW: INDUSTRIAL APPEARANCE WHERE NOT EXPOSED TO VIEW. ALL MEMBERS TO HAVE EXTERIOR GLUE AND HAVE AN APPROVED GRADE STAMP. CAMBER AS SHOWN ON STRUCTURAL DRAWINGS.

FRAMING LUMBER: STANDARDS. EACH PIECE SHALL BEAR THE GRADE TRADEMARK OF THE WEST COAST LUMBER INSPECTION BUREAU (WCLIB), WESTERN WOOD PRODUCTS ASSOCIATION (WWPA), OR OTHER AGENCY ACCREDITED BY THE AMERICAN LUMBER STANDARD COMMITTEE (ALSC) TO GRADE UNDER ALSC CERTIFIED GRADING RULES.

#### <u>SPECIES AND GRADE</u> (BASE DESIGN VALUE)

- 6x BEAMS AND HEADERS. "DOUG FIR-LARCH" NO. 1 (Fb=1350 PSI, Fv=170 PSI) 2. 2x TO 4x JOISTS, PURLINS AND HEADERS. "DOUG FIR-LARCH" NO. 2 (Fb=900 PSI, Fv=180 PSI) OR "HEM-FIR"
- NO. 1 (Fb=975 PSI, Fv=150 PSI)
- 6x POSTS AND COLUMNS. "DOUG FIR-LARCH" NO. 1 (Fc=1000 PSI) EXTERIOR STUDS, INTERIOR BEARING WALLS AND 4x COLUMNS. "DOUG FIR-LARCH" NO. 2 (Fb= 900 PSI,
- Fc=1350 PSI) OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fc=1350 PSI). INTERIOR NON-BEARING STUD WALLS. "DOUG FIR-LARCH" NO. 2 (Fb=900 PSI, Fc=1350 PSI) OR "HEM-FIR"
- NO. 1 (Fb=975 PSI, Fc=1350 PSI)
- 2x & 3x T&G DECKING: "DOUG FIR-LARCH" COMMERCIAL (Fb=1450 PSI, E=1700 KSI) THE MINIMUM GRADE OF ALL OTHER STRUCTURAL FRAMING. "DOUG FIR-LARCH" NO. 2 (Fb= 900 PSI,
- Fc=1350 PSI), OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fc=1350 PSI). 8. UTILITY & STANDARD GRADES NOT PERMITTED.

STRUCTURAL COMPOSITE LUMBER (SCL): SHALL BE MANUFACTURED BY REDBUILT LLC., OR PRE-APPROVED EQUAL IN ACCORDANCE WITH APPROVED SHOP AND INSTALLATION DRAWINGS CONFORMING TO A CURRENT **EVALUATION REPORT.** 

## MIINIMUM DESIGN VALUES

- 2x SCL: Fb = 1700 PSI, Fv = 285 PSI, E = 1300 KSI 1-3/4" SCL: Fb = 2600 PSI, Fv = 285 PSI, E = 1800 KSI
- 3-1/2" SCL: Fb = 2900 PSI, Fv = 285 PSI, E = 2000 KSI
- 5-1/4" SCL: Fb = 2900 PSI, Fv = 285 PSI, E = 2000 KSI RIMBOARD: APA/EWS PERFORMANCE RATED RIM (PRR-401) 1-1/4" MINIMUM THICKNESS

MEMBERS HAVE BEEN DESIGNED TO SERVICEABILITY AND OTHER PERFORMANCE BASED REQUIREMENTS, WHICH MAY EXCEED MINIMUM DESIGN LOADS AND CODE REQUIREMENTS. SUBSTITUTIONS MUST MEET OR

EXCEED MOMENT. SHEAR, AND STIFFNESS OF THOSE MEMBERS SPECIFIED AT THE SAME DEPTH AND SPACING.

## PRESERVATIVE TREATED WOOD REQUIREMENTS:

CBA & CA: COPPER AZOLE

## TREATMENTS OTHER THAN THOSE LISTED BELOW ARE NOT PERMITTED:

		APPLICATION	SPECIFIED MATERIAL	PRESERVATIVE TREATMENT (1)	CONNECTORS & FASTENERS (2)(3)
<b>\</b>		FOUNDATION SILL PLATES, TOP PLATES & LEDGERS ON	2x, 4x, 6x (FIR), OR GLULAM (SP)	SBX	GALV (G60)
IRE DRY	DR	CONCRETE OR MASONRY WALLS (4)		ACQ, CBA, CA	GALV (G185)
้อรเ	EXPOSURE :T	FRAMING, DECKING,	2x, & 4x (FIR)	ACQ, CBA, CA	GALV (G185)
EXP		POSTS & LEDGERS	2x, & 4x (CEDAR)	NONE	GALV (G90)
	WET	BEAMS & COLUMNS	6x (FIR), OR GLULAM (SP)	ACQ, CBA, CA	GALV (G185)
			6x OR GLULAM (CEDAR)	NONE	GALV (G90)

1. CCA: CHROMATED COPPER ARSENATE NOT PERMITTED FIR: DOUG-FIR OR HEM-FIR SBX: DOT SODIUM BORATE SP: SOUTHERN PINE ACQ: ALKALINE COPPER QUAT

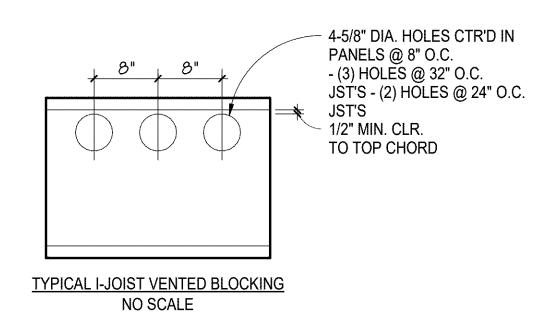
- CONNECTORS: JOIST HANGERS, STRAPS, FRAMING CONNECTORS, COLUMN CAPS AND BASES, ETC. FASTENERS: MACHINE BOLTS, ANCHOR BOLTS AND LAG SCREWS WITH ASSOCIATED PLATE WASHERS AND NUTS. NAILS, SPIKES, WOOD SCREWS, ETC.
- G60, G90 & G185 PER ASTM A653 FOR COLD-FORMED STEEL CONNECTORS. BATCH/POST HOT-DIP GALVANIZED PER ASTM A123 FOR CONNECTORS AND ASTM A153 STRUCTURAL STEEL CONNECTORS. HOT-DIP GALVANIZED PER ASTM A153 FOR FASTENERS OR MECHANICALLY GALVANIZED FASTENERS PER ASTM B695, CLASS 55 OR GREATER.
- 4. AT CONTRACTORS OPTION, LEDGERS AND TOP PLATES A MINIMUM OF 8 FEET ABOVE GRADE ON CONCRETE OR MASONRY WALLS MAY BE UN-TREATED IF COMPLETELY SEPARATED FROM THE WALL BY A SELF ADHERING ICE & WATER SHIELD BARRIER (40 MIL MINIMUM).

GENERAL REQUIREMENTS: PROVIDE MINIMUM NAILING PER IBC TABLE 2304.10.1 OR MORE, AS OTHERWISE SHOWN. STAGGER ALL NAILING TO PREVENT SPLITTING OF WOOD MEMBERS. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE TREATED WITH THE EXCEPTION OF INTERIOR CONCRETE TOPPINGS ON WOOD FLOOR SYSTEMS. HOLES AND CUTS IN 3x OR 4x PLATES SHOULD BE TREATED WITH A 9% SOLUTION OF COPPER NAPHTHENATE. BOLT HOLES IN WOOD MEMBERS SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER. PROVIDE CUT WASHERS WHERE BOLT HEADS, NUTS AND LAG SCREW HEADS BEAR ON WOOD. PROVIDE A MINIMUM 3"x3"x0.229" PLATE WASHER ON ALL ANCHOR BOLTS WHICH CONNECT MUD SILLS TO FOUNDATION. DO NOT NOTCH OR DRILL STRUCTURAL MEMBERS, EXCEPT AS ALLOWED BY IBC SECTIONS 2308.4.2.4, 2308.5.9, 2308.5.10 AND 2308.7.4 OR AS RESTRICTED BY PLANS OR DETAILS OR AS APPROVED PRIOR TO INSTALLATION. REFER TO PRESERVATIVE TREATED WOOD REQUIREMENTS IN THESE GENERAL NOTES FOR GALVANIZING REQUIREMENTS FOR CONNECTORS AND FASTENERS.

<u>FRAMING CONNECTORS</u>: SHALL CONFORM TO CURRENT EVALUATION REPORT AND BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, SAN LEANDRO, CA., OR PRE-APPROVED EQUAL. PROVIDE MAXIMUM SIZE AND QUANTITY OF NAILS OR BOLTS PER MANUFACTURER, EXCEPT AS NOTED OTHERWISE. PROVIDE LEAD HOLES AS REQUIRED TO PREVENT SPLITTING OF WOOD MEMBERS. REFER TO PRESERVATIVE TREATED WOOD REQUIREMENTS IN THESE GENERAL NOTES FOR GALVANIZING REQUIREMENTS FOR CONNECTORS AND

LAG SCREWS: SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1. LAG SCREWS SHALL BE OF A DIAMETER INDICATED ON DRAWINGS WITH A MINIMUM OF 8x DIA. EMBEDMENT IN SUPPORTING MEMBER UNLESS NOTED OTHERWISE. CLEARANCE HOLE FOR THE SHANK SHALL BE THE SAME DIAMETER AS THE SHANK AND THE SAME DEPTH OF PENETRATION AS THE UNTHREADED PORTION OF THE SHANK. THE LEAD HOLE FOR THE THREADED PORTION SHALL HAVE A DIAMETER EQUAL TO 60 TO 75 PERCENT OF THE SHANK DIAMETER AND A LENGTH EQUAL TO AT LEAST THE LENGTH OF THE THREADED PORTION. THE THREADED PORTION OF THE SCREW SHALL BE INSERTED IN ITS LEAD HOLE BY TURNING WITH A WRENCH. SOAP OR OTHER LUBRICANT SHALL BE USED ON THE SCREWS OR IN THE LEAD HOLE TO FACILITATE INSERTION AND PREVENT DAMAGE TO THE SCREW. LAG SCREWS SHALL NOT BE DRIVEN WITH A HAMMER. REFER TO <u>PRESERVATIVE TREATED WOOD REQUIREMENTS</u> IN THESE GENERAL NOTES FOR GALVANIZING REQUIREMENTS FOR CONNECTORS AND FASTENERS.

I-JOISTS: SHALL BE MANUFACTURED BY REDBUILT LLC, OR PRE-APPROVED EQUAL IN ACCORDANCE WITH APPROVED SHOP AND INSTALLATION DRAWINGS. MEMBERS SHALL BE DESIGNED UNDER THE DIRECT SUPERVISION OF A STRUCTURAL ENGINEER LICENSED IN THE STATE OF PROJECT. THE ENTIRE I-JOIST ASSEMBLY SHALL BE AS APPROVED BY CURRENT EVALUATION REPORT. MEMBERS SHALL BE DESIGNED TO CARRY THE LOADS LISTED IN THE DESIGN CRITERION AND ANY ADDITIONAL LOADS INDICATED ON THE FRAMING PLANS AND DETAILS. THE ENGINEER SHALL ASSUME ALL RESPONSIBILITY FOR THE WORK OF ALL SUBORDINATES INVOLVED IN THE PREPARATION OF THE PLACEMENT PLANS AND DESIGN DRAWINGS. I-JOISTS SHALL BE PROVIDED TO COMPLETE THE ROOF AND/OR FLOOR FRAMING FROM THE SHEATHING TO THE SUPPORTING MEMBERS BELOW. MEMBER DESIGNATIONS ON PLANS ARE FOR TYPICAL UNIFORMLY LOADED CONDITIONS. MANUFACTURER SHALL PROVIDE ADDITIONAL MEMBERS AS REQUIRED TO SUPPORT SPECIAL LOADING CONDITIONS INDICATED ON DRAWINGS. PROVIDE SHOP AND INSTALLATION DRAWINGS AND CALCULATIONS PRODUCED UNDER THE SUPERVISION OF AND BEARING THE STAMP OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF PROJECT. DETAIL DRAWINGS TO INDICATE MEMBER TYPES, SIZE SPACING, BRIDGING, BLOCKING, CONNECTIONS, ANCHORING, BEARING PLATE AND OTHER PERTINENT DETAILS PROVIDE 1 1/2" DIA. OPEN KNOCKOUTS AT 12" O.C. ON ALL ROOF I-JOISTS.



MEMBER DESIGN CALCULATIONS SHALL BE PROVIDED FOR STANDARD LOADING ALONG WITH DESIGN CHECKS FOR SPECIAL LOADING CONDITIONS WHICH INCLUDE FREE BODY DIAGRAMS, LOADING BREAK DOWN, DESCRIPTION OF LOADS (I.E. MECH UNIT, SUSPENDED WALL, ETC.) AND THE RATIONALE FOR LOADING DISTRIBUTION ON MULTIPLE MEMBERS. SUBMITTAL SHALL ALSO PROVIDE ANY DOCUMENTATION NECESSARY TO INTERPRET DATA INDICATED ON CALCULATIONS.

MEMBERS HAVE BEEN DESIGNED TO MEET SERVICEABILITY AND OTHER PERFORMANCE BASED REQUIREMENTS, WHICH MAY EXCEED MINIMUM DESIGN LOADS AND CODE REQUIREMENTS. SUBSTITUTIONS MUST MEET OR EXCEED MOMENT, SHEAR, AND STIFFNESS OF THOSE MEMBERS SPECIFIED AT THE SAME DEPTH AND SPACING.

REFER TO THE <u>FRAMING CONNECTORS</u> SECTION OF THESE GENERAL NOTES FOR REQUIREMENTS PLACED UPON CONNECTOR HARDWARE SPECIFIED BY TRUSS ENGINEER AND/OR PROVIDED BY TRUSS MANUFACTURER

SPRINKLER LINE ATTACHMENTS SHALL CONFORM TO NFPA 13 AND COMMERCIAL PUBLICATION "SPRINKLER SYSTEM INSTALLATION WITH GUIDELINES FOR REDBUILT OPEN-WEB TRUSSES AND I-JOISTS". LOADS HUNG FROM JOIST NOT SPECIFICALLY IDENTIFIED ON STRUCTURAL DRAWINGS SHALL NOT EXCEED 30 POUNDS AT ANY ONE POINT. NOR SHALL TOTAL LOADS IN POUNDS ON ANY ONE JOIST EXCEED 8 TIMES THE JOIST SPAN IN FEET, UNLESS DETAILED OTHERWISE ON THE DRAWINGS. ATTACHMENT OF LOADS EXCEEDING 90 POUNDS SHALL BE APPROVED PRIOR TO INSTALLATION. DO NOT NOTCH OR DRILL THRU TRUSS MEMBERS

## MISCELLANEOUS:

PRE-APPROVED SUBSTITUTIONS: SUBSTITUTIONS MAY BE ALLOWED ONLY IF THEY MEET THE REQUIREMENTS OF THESE GENERAL NOTES AND THE SPECIFICATIONS. AND IF COMPLETE WRITTEN ENGINEERING DATA FOR EACH CONDITION REQUIRED FOR THIS PROJECT IS PROVIDED TO THE STRUCTURAL ENGINEER TWO WEEKS PRIOR TO BID DATE AND APPROVED IN WRITTEN ADDENDA BY THE ARCHITECT. DATA IS TO INDICATE CODE BASIS BY YEAR, AUTHORITY FOR STRESSES AND STRESS INCREASES, IF ANY, AND AMOUNT OF EXPECTED DEFLECTION FOR FLEXURAL MEMBERS UNDER (1) TOTAL LOAD AND (2) LIVE LOAD ONLY. ALL INCREASED COSTS IN MECHANICAL, SPRINKLER, ELECTRICAL OR GENERAL INSTALLATION AND ANY ARCHITECTURAL OR STRUCTURAL REDESIGN RESULTING FROM SUBSTITUTION SHALL BE BORNE BY THE GENERAL CONTRACTOR

## SHOP DRAWINGS/SUBMITTALS

THE FOLLOWING SHOP DRAWINGS/SUBMITTALS SHALL BE PROVIDED FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION OR DELIVERY.

		STRUCTURAL ENGR.	BLDG. DEPT.
1.	CONCRETE MIX DESIGNS	X	Х
2.	REINFORCING STEEL SHOP DRAWINGS	X	
3.	VENEER ANCHORAGE SYSTEMS	X	X
4.	STRUCTURAL STEEL	X	X
5.	MISCELLANEOUS STEEL	X	X
3.	GLU-LAMINATED MEMBERS	X	X
7.	STRUCTURAL COMPOSITE LUMBER	X	X
8.	WOOD I-JOISTS	X	X
9.	CONTRACTOR'S STATEMENT OF RESPONSIBILITY	X	X

## **DEFERRED SUBMITTALS**

THE FOLLOWING ARE NOT INCLUDED WITH THE BUILDING PERMIT DRAWINGS AND SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL AS A DEFERRED SUBMITTAL SUBMITTALS SHALL BEAR THE SEAL OF AN ENGINEER LICENSED IN THE STATE OF THE PROJECT AS NOTED.

	ENGINEER STAMP REQUIRED
I-JOISTS	SE

SPECIAL INSPECTION: SPECIAL INSPECTION SHALL BE PROVIDED BY AN INDEPENDENT TESTING LABORATORY PER THE REQUIREMENTS OF IBC CHAPTER 17 AND THE LOCAL BUILDING OFFICIAL OR APPLICABLE JURISDICTION AND THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR SHALL SUBMIT INSPECTION REPORTS AND A FINAL SIGNED REPORT TO THE BUILDING OFFICIAL FOR THE ITEMS LISTED IN THE QUALITY ASSURANCE/SPECIAL INSPECTION SECTION:

architects

Jurisdiction Stamp Area

71 Columbia Street, Suite 500 Seattle, Washington 98104 T (206) 340 9500 F (206) 340 9519

CIVIL ENGINEER AND LANDSCAPE MacKay Sposito 1325 SE Tech Center Drive, Suite 140 Vancouver, WA 98683 T (360) 695 3411

STRUCTURAL ENGINEER PCS Structural Solutions One Main Place 101 SW Main Street, Suite 280 Portland, OR 97204 T (503) 232 3746

BCE Engineers 6021 12th St E, Suite 200 Fife, WA 98424 T (253) 922-0446 **ELECTRICAL ENGINEER** BCE Engineers

MECHANICAL ENGINEER

6021 12th St E, Suite 200 Fife, WA 98424 T (253) 922-0446 ACOUSTIC 4100 194th St., SW, Ste. 400

Lynnwood, WA 98036

T (206) 667 0555

COST CONSULTANT Rider Levett Bucknall (RLB) Brewery Block 2 1120 NW Couch Street, Suite 730 Portland, OR 97209 T (503) 226 2730

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REVISIONS DATE

Vancouver School District FRANKLIN

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ISSUE DATE:

Stamp Area

**GENERAL NOTES** 

#### **STATEMENT OF SPECIAL INSPECTIONS**:

SPECIAL INSPECTION: SPECIAL INSPECTION SHALL BE PROVIDED PER THE REQUIREMENTS OF IBC SECTION 1704 AND 1705 AND AS NOTED HEREIN.

STRUCTURAL SYSTEM	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	COMMENTS	REFERENCES
SOILS	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY		Х		IBC 1705.6
	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL		Χ		
	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS		X		
	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X			
	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY		Х		
STEEL CONSTRUCTION	MATERIAL VERIFICATION OF STRUCTURAL STEEL A. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360		Х	MANUFACTURER TO PROVIDE CERTIFIED MILL TEST REPORTS	AISC 360 CHAPTER N5 AISC 341 CHAPTER J6
	B. MANUFACTURER'S CERTIFIED MILL TEST REPORTS		Х		AISC 360 CHAPTER N5
	MATERIAL VERIFICATION OF WELD FILLER MATERIALS A. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATIONS LISTED IN GENERAL NOTES B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE		X X	MANUFACTURER TO PROVIDE CERTIFICATE OF COMPLIANCE	AISC 300 CHAPTER NS
	INSPECTION OF WELDING A. COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS B. MULTI-PASS FILLET WELDS C. SINGLE-PASS FILLET WELDS > 5/16" D. PLUG AND SLOT WELDS E. SINGLE-PASS FILLET WELDS ≤ 5/16"	X X X X	X	SPECIAL INSPECTIONS IN THIS SECTION ARE WAIVED WHERE FABRICATION IS PERFORMED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED IN ACCORDANCE WITH IBC SECTION 1704.2.5	AISC 360 CHAPTER N5 AISC 341 CHAPTER J6 AWS D1.1
CONCRETE	REINFORCING STEEL AND PLACEMENT		Х	SPECIAL INSPECTIONS NOT REQUIRED FOR THE FOLLOWING CONDITIONS:	ACI 318: CH 20, 25.2, 25.3, 26.6-1 TO 26.6-3, IBC 1908.4
	ANCHORS CAST IN CONCRETE-PRIOR TO AND DURING PLACEMENT OF CONCRETE		Х	NON-STRUCTURAL SLAB ON GRADE	ACI 318: 17.8.2 AISC 360 SECTION N7
	VERIFY USE OF REQUIRED DESIGN MIX		X		ACI 318, CH 19
	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X			ASTM C172, C31 ACI 318: 26.4, 26.12 IBC 1908.10
	MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		Χ		ACI 318: 26.5.3 TO 26.5.5 IBC 1908.9
	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		Х		ACI 318: 26.11.1.2(b)
	MATERIAL VERIFICATION OF REINFORCEMENT STEEL FOR ASTM A615 REINFORCING		Х	MANUFACTURER SHALL PROVIDE MILL TEST REPORTS. CONTINUOUS INSPECTION FOR ALL WELDS GREATER THAN 5/16" FILLET. PERIODIC INSPECTION FOR FILLET WELD 5/16" AND SMALLER	ACI 318: 26.6.4 AWS D1.4 IBC 1705.3.1
	TESTING OF MATERIALS		Х		IBC 1705.3.2
	ANCHORS POST-INSTALLED IN HARDENED CONCRETE (MECHANICAL ANCHORS AND ADHESIVE ANCHORS INSTALLED DOWNWARD)		X	PERIODIC INSPECTION TO INCLUDE A QUANTITY OF 10% WITH A MINIMUM OF (5) ANCHORS INSPECTED PER INSTALLER ON A DAILY BASIS	ACI 318: 17.8.2 MFR EVAL REPORT MFR PUBLISHED INSTALLATION INSTRUCTIONS
	ANCHORS POST-INSTALLED IN HARDENED CONCRETE (MECHANICAL ANCHORS AND ADHESIVE ANCHORS INSTALLED HORIZONTALLY OR UPWARDLY INCLINED)	X			ACI 318: 17.8.2 MFR EVAL REPORT MFR PUBLISHED INSTALLATION INSTRUCTIONS
WOOD FRAMING	SHEAR WALL NAILING		Х	SPECIAL INSPECTION NOT REQUIRED FOR FASTENER SPACING > 4" O.C.	IBC 1705.11.1, 1705.12.2, 1705.5
	DIAPHRAGM NAILING		Х	SPECIAL INSPECTION NOT REQUIRED FOR FASTENER SPACING > 4" O.C.	IBC 1705.11.1, 1705.12.2, 1705.5
	NAILING, BOLTING, AND ANCHORAGE OF COMPONENTS THAT ARE PART OF DRAG STRUTS, BRACES AND HOLD-DOWNS THAT ARE PART OF THE SEISMIC RESISTING SYSTEM		Х		IBC 1705.11.1, 1705.12.2
SUSPENDED CEILINGS	ANCHORAGE AND SEISMIC BRACING		Х		ASCE 7-10, APPENDIX 11A
ANCHORED VENEER	INSPECTION PROGRAM SHALL VERIFY:  1. SIZE, TYPE OF VENEER ANCHORS  2. SIZE, GRADE OF JOINT REINF.  3. PROPORTIONS OF MORTAR  4. CONSTRUCTION OF MORTAR JOINTS  5. INSTALLATION OF TIES		X X X X	VERIFICATION AT BEGINNING OF CONSTRUCTION	IBC 1705.12.5, 1705.4 TMS 402 / ACI 530 / ASCE 5

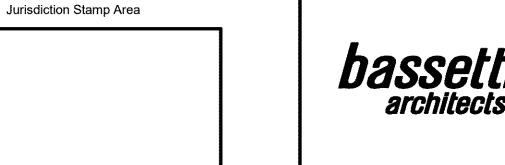
TESTING AND SPECIAL INSPECTION REPORTS SHALL BE PREPARED FOR EACH INSPECTION ITEM ON A DAILY BASIS WHENEVER WORK IS PERFORMED ON THAT ITEM. REPORTS SHALL BE DISTRIBUTED TO OWNER, CONTRACTOR, BUILDING OFFICIAL, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.

STRUCTURAL OBSERVATIONS SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD OR DESIGNATED REPRESENTATIVE IN ACCORDANCE WITH IBC 1704.6. STRUCTURAL OBSERVATION SHALL BE PERFORMED AS FOLLOWS:

- » PERIODIC VISUAL OBSERVATION OF STRUCTURAL SYSTEMS FOR GENERAL CONFORMANCE TO CONSTRUCTION DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES.
- » REVIEW OF TESTING AND INSPECTION REPORTS.
- » REPORTS SHALL BE PREPARED FOR EACH SITE VISIT AND SHALL BE DISTRIBUTED TO ARCHITECT.

GENERAL CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER PRIOR TO COMMENCEMENT OF WORK. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL INCLUDE ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTION.

	ABBREVIATI	ולו אט	
0	AT	HGR	HANGER
A.B.	ANCHOR BOLT	HORIZ.	HORIZONTAL
ADD'L	ADDITIONAL	HSS	HOLLOW STRUCTURAL SECTION
A.F.F.	ABOVE FINISH FLOOR	HT	HEIGHT
ALT.	ALTERNATE	INT.	INTERIOR
ARCH.	ARCHITECTURAL	JST	JOIST
BLD'G	BUILDING	J	JOINT
BLK'G	BLOCKING	L	ANGLE
ВМ	BEAM	L.L.	LIVE LOAD
B.O.F.	BOTTOM OF FOOTING	LLH	LONG LEG HORIZONTAL
ВОТ.	ВОТТОМ	LLV	LONG LEG VERTICAL
BRG	BEARING	LOC.	LOCATION
BTWN	BETWEEN	LSL	LAMINATED STRAND LUMBER
B.V.	BUILT UP	LVL	LAMINATED VENEER LUMBER
(C= )	CAMBER	MAX.	MAXIMUM
CANT.	CANTILEVER	M.B.	MACHINE BOLT
C.F.S.	COLD-FORMED STEEL	MECH.	MECHANICAL
C.J.	CONTROL/CONSTRUCTION JOINT	MEZZ.	MEZZANINE
<u> </u>	CENTERLINE	MFR	MANUFACTURER
CLR.	CLEARANCE	MIN.	MINIMUM
CMU	CONCRETE MASONRY UNIT	MISC.	MISCELLANEOUS
COL.	COLUMN	MTL	METAL
CONC.	CONCRETE	N.F.	NEAR FACE
CONN.	CONNECTION	N.S.	NEAR SIDE
CONST.	CONSTRUCTION	NTS	NOT TO SCALE
CONT.	CONTINUOUS	O.C.	ON CENTER
CONTR.	CONTRACTOR	OPN'G	OPENING
COORD.	COORDINATE	OPP.	OPPOSITE
C.P.	COMPLETE PENETRATION	P.A.F.	POWDER ACTUATED FASTENER
CTR'D	CENTERED	PERP.	PERPENDICULAR
C.Y.	CUBIC YARD	P	PLATE
DBL.	DOUBLE	P.P.	PARTIAL PENETRATION
D.F.	DOUGLAS FIR	P.P.T.	PRESERVATIVE PRESSURE TREATED
DIA. OR Φ	DIAMETER	P.S.F.	POUNDS PER SQUARE FOOT
DIAG.	DIAGONAL	PSL	PARALLAM
DIM.	DIMENSION	P.T.	POST TENSION
D.L.	DEAD LOAD	PW.	PLYMOOD
DWG	DRAWING	REINF.	REINFORCING
DWL	DOWEL	REQ'D	REQUIRED
(E)	EXISTING	SCHED.	SCHEDULE
EA.	EACH	S.C.L.	STRUCTURAL COMPOSITE LUMBE
E.F.	EACH FACE	SHT'G	SHEATHING
EL.	ELEVATION	SIM.	SIMILAR
ELEV.	ELEVATOR	S.O.G.	SLAB ON GRADE
ENGR.	ENGINEER	SQ.	SQUARE
EQ.	EQUAL	STD	STANDARD
E.W.	EACH WAY	STIFF.	STIFFENER
EXP.	EXPANSION	STL	STEEL
EXT.	EXTERIOR	STRUCT.	STRUCTURAL
FDN	FOUNDATION	T <b>#</b> B	TOP & BOTTOM
F.F.	FAR FACE	T# <i>G</i>	TONGUE AND GROOVE
FLR	FLOOR	THR'D	THREADED
F.O.M.	FACE OF MASONRY	T.O.F.	TOP OF FOOTING
F.O.S.	FACE OF STUD	T.O.S.	TOP OF STEEL
FRM'G	FRAMING	TRT'D	TREATED
F.R.T.	FIRE RETARDANT TREATED	TYP.	TYPICAL
F.S.	FAR SIDE	U.N.O.	UNLESS NOTED OTHERWISE
FTG	FOOTING	U.T.	ULTRASONIC TESTED
			VERTICAL
GALV	GALVANIZED	VERT.	
GALV.	GALVANIZED	M/	WITH DOINT
<i>-</i> ·	GLULAM	W.P.	WORK POINT
GL.	CD A DE		1.0-1-1.1-
GL. GR. GWB	GRADE GYPSUM WALL BOARD	MT M.M.R.	WEIGHT WELDED WIRE REINFORGING



71 Columbia Street, Suite 500 Seattle, Washington 98104 T (206) 340 9500 F (206) 340 9519

CIVIL ENGINEER AND LANDSCAPE MacKay Sposito 1325 SE Tech Center Drive, Suite 140 Vancouver, WA 98683 T (360) 695 3411

STRUCTURAL ENGINEER PCS Structural Solutions One Main Place 101 SW Main Street, Suite 280 Portland, OR 97204 T (503) 232 3746

MECHANICAL ENGINEER BCE Engineers 6021 12th St E, Suite 200 Fife, WA 98424 T (253) 922-0446

ELECTRICAL ENGINEER BCE Engineers 6021 12th St E, Suite 200 Fife, WA 98424 T (253) 922-0446 ACOUSTIC

Stantec

4100 194th St., SW, Ste. 400 Lynnwood, WA 98036 T (206) 667 0555 COST CONSULTANT Rider Levett Bucknall (RLB) Brewery Block 2

1120 NW Couch Street, Suite 730

Portland, OR 97209

T (503) 226 2730 ONE INCH





# REVISIONS DATE

Vancouver School District FRANKLIN **ELEMENTARY** SCHOOL **ADDITION** 

GENERAL

9'-0"

T.O.F. = 98'-6"

TYP. AT PERIMETER

U.N.O.

SEE ARCH. FOR STEM WALL TRANSITION ——

T.O.F. = 98'-6"

T.O.F. = 96'-0"

T.O.F. = 98'-6"

CONNECT NEW
TO (E) FTG
PER 1 - TYP.

FRAME NEW
OPNG'S &
PROVIDE STL
LINTEL ANGLES

PER 1 \$ 6 54.01

33'-0"

5 53.01

- 4" CONC. S.O.G. W/ FIBROUS REINF. (1.5#/C.Y.) - SEE

2'-8 1/8"-



12.

#### FOUNDATION AND GRADE LEVEL FRAMING NOTES

- 1. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- 2. REFERENCE ELEVATION 100'-0" = 190.00'.
- 3. TOP OF SLAB = 100'-0" UNLESS NOTED OTHERWISE.

INDICATES WOOD STUD WALL WITH STUDS SPACED AT 16" ON CENTER MAXIMUM UNLESS NOTED OTHERWISE. PROVIDE 1/2" WOOD SHEATHING AT ALL EXTERIOR WALLS NAILED WITH 8d AT 6" ON CENTER AT ALL PANEL EDGES (PROVIDE 2x BLOCKING AT UNSUPPORTED PANEL EDGES) AND 8d AT 12" ON CENTER AT INTERMEDIATE FRAMING TYPICAL UNLESS NOTED OTHERWISE. SEE NOTE #11 FOR ADDITIONAL SHEAR WALL NAILING.

> - INDICATES CONCRETE WALL FOOTING 2'-0" UNLESS NOTED OTHERWISE. SEE \$3.02 FOR TYPICAL FOOTING DETAILS.

- INDICATES MASONRY VENEER. FOR ATTACHMENT OF VENEER TO BACKING - SEE GENERAL NOTES. FOR VENEER LINTEL ANGLES AT OPENINGS - SEE 6/S4.01.

INDICATES CONCRETE SPREAD FOOTING - SEE 8/53.02 FOR SCHEDULE.

INDICATES HOLLOW STRUCTURAL SECTION COLUMNS ORIGINATING AT FOUNDATION.

7'-0"

(E) FACE OF CONC.

- (E) S.O.G. PATCH PER 9

 $\downarrow$   $\bigcirc$ 

27'-6"

10 53.02

SET COL. SO
COL. CAP DOES
NOT CONFLICT
W/ WALL FINISH

INDICATES WOOD STUD BUILT-UP COLUMN - SEE 2/S4.01 FOR TYPICAL INDICATES SPECIAL BUILT-UP WOOD STUD COLUMN REQUIREMENTS UNDER HEADER. FOR TYPICAL FRAMING REQUIREMENTS AT OPENING IN STRUCTURAL WALLS - SEE 1/S4.01 FOR TYPICAL DETAIL.

INDICATES SPECIAL WOOD STUD WALL TYPE - SEE 4/S4.Ø1 FOR SCHEDULE.

INDICATES HOLDOWN - SEE 1/S4.03 FOR SCHEDULE.

13. FOR TYPICAL CONCRETE SLAB-ON-GRADE DETAILS - SEE S3.01.

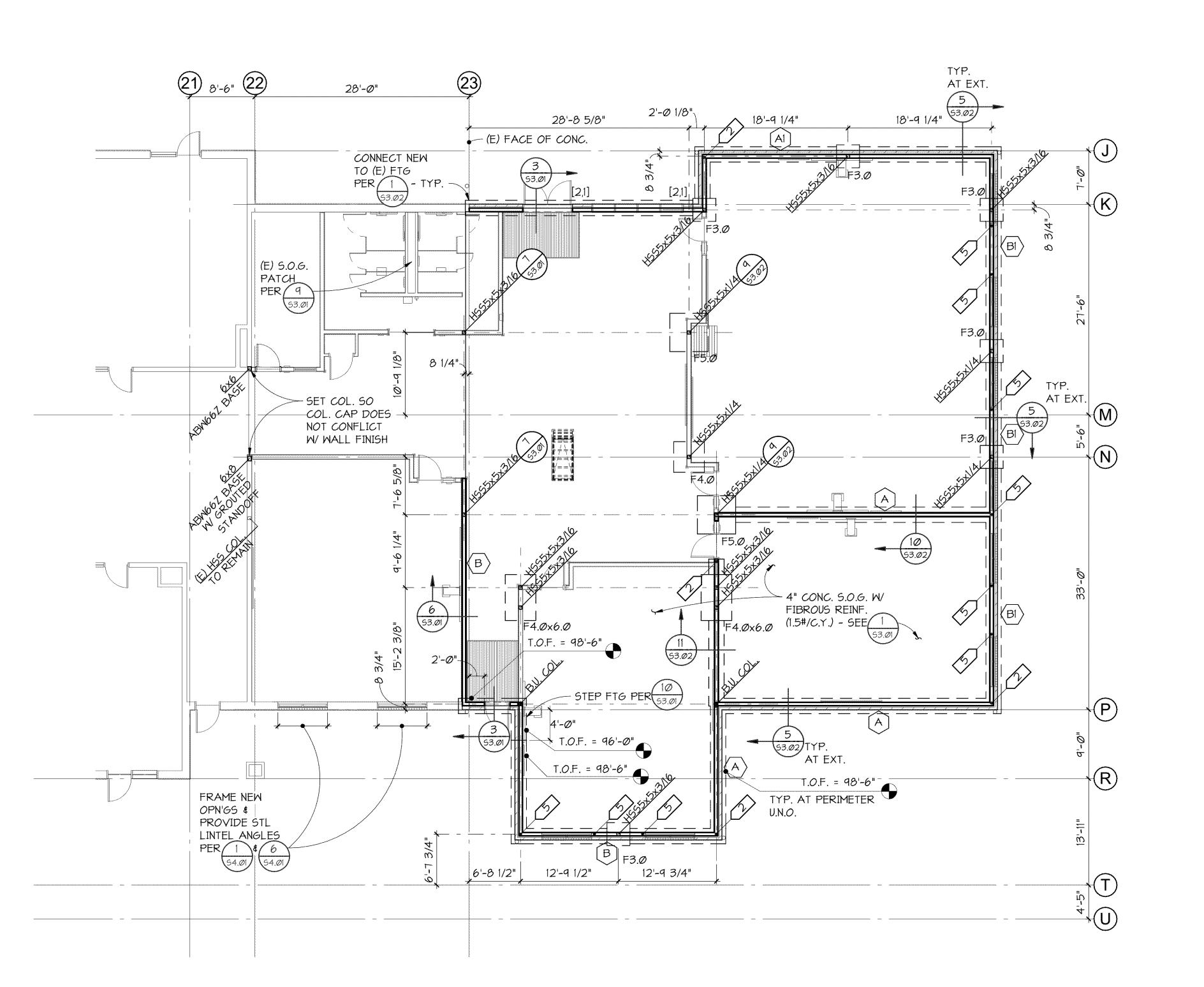
14. FOR TYPICAL PLACEMENT OF STEM WALL REINFORCEMENT AND FOUNDATION CONSTRUCTION JOINTS - SEE SHEET S3.02.

15. FOR TYPICAL EXCAVATION LIMITATIONS IN THE PROXIMITY OF FOUNDATIONS - SEE DETAIL 2/S3.02.

16. FOR TYPICAL VERTICAL PIPE PENETRATIONS IN STEM WALLS - SEE 5/S4.02.

17. NON-STRUCTURAL STUD WALLS ARE NOT SHOWN OR SHOWN SCREENED. FOR LOCATION - SEE ARCHITECTURAL DRAWINGS. FOR BRACING AT TOPS OF WALLS - SEE SHEET S4.06.

18. STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THIS RESPONSIBILITY INCLUDES BUT IS NOT LIMITED TO TEMPORARY SHORING AND BRACING OF THE EXISTING AND NEW STRUCTURE. SEE THE STRUCTURAL GENERAL NOTES FOR FURTHER LANGUAGE AND REQUIREMENTS.







71 Columbia Street, Suite 500 Seattle, Washington 98104 T (206) 340 9500 F (206) 340 9519

CIVIL ENGINEER AND LANDSCAPE MacKay Sposito 1325 SE Tech Center Drive, Suite 140 Vancouver, WA 98683 T (360) 695 3411

STRUCTURAL ENGINEER PCS Structural Solutions One Main Place 101 SW Main Street, Suite 280 Portland, OR 97204 T (503) 232 3746

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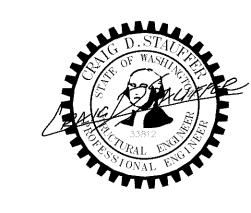
**ELECTRICAL ENGINEER** BCE Engineers 6021 12th St E, Suite 200 Fife, WA 98424 T (253) 922-0446

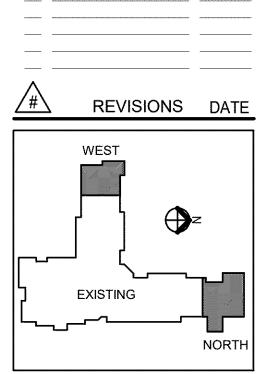
ACOUSTIC 4100 194th St., SW, Ste. 400 Lynnwood, WA 98036 T (206) 667 0555

COST CONSULTANT Rider Levett Bucknall (RLB) Brewery Block 2 1120 NW Couch Street, Suite 730 Portland, OR 97209 T (503) 226 2730

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Vancouver School District FRANKLIN **ELEMENTARY** SCHOOL **ADDITION** 

5206 NW Franklin St, Vancouver, WA 98663

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FOUNDATION AND GRADE LEVEL FRAMING **PLANS** 

S2.01

8

NOTE: FOR STEEL LINTEL ANGLES - SEE 6

9'-0"



11 12

27'-6"

TYP. ALL
A)SIDES AROUND

MEZZANINE

- DBL. JST

SIMPSON

COL. CAP

ECCQ5-65D52.5

- SIMPSON MSTI60 STRAP W/ 2x FLAT

BLK'G IN (E) WALL

5/8×161/2 GL. 8 54.04

FRM'G PER (54.01)

B

33'-0"

5%×16½ GL.

SIMPSON HUC5.125/16 HGR —

11%" RED-165 @ 16" O.C. —

TYP. AT 8 CONT. COL. 64.05

CEILING 7 FRM'G PER (54.01)

SIMPSON ECCQ5-8SDS2.5 COL. CAP -

(E) MEZZANINE FRM'G -

14)

# FLOOR FRAMING NOTES

1. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. MEZZANINE FINISH FLOOR = 110'-10" UNLESS NOTED OTHERWISE.

INDICATES WOOD STUD WALL WITH STUDS SPACED AT 16" ON CENTER MAXIMUM UNLESS NOTED OTHERWISE. PROVIDE 1/2" WOOD SHEATHING AT ALL WALLS NAILED WITH 8d AT 6" ON CENTER AT ALL PANEL EDGES (PROVIDE 2x BLOCKING AT UNSUPPORTED PANEL EDGES) AND 8d AT 12" ON CENTER AT INTERMEDIATE FRAMING TYPICAL UNLESS NOTED OTHERWISE. SEE NOTE #9 FOR ADDITIONAL SHEAR WALL NAILING.

INDICATES MASONRY VENEER. FOR ATTACHMENT OF VENEER TO BACKING - SEE GENERAL NOTES. FOR VENEER LINTEL ANGLES AT OPENINGS - SEE 6/S4.Ø1.

INDICATES CAMBER FOR GLULAM BEAMS. C=0" UNLESS NOTED OTHERWISE.

INDICATES HOLLOW STRUCTURAL SECTION COLUMNS ORIGINATING AT FLOOR LEVEL.

INDICATES TYPE OF CONTINUOUS COLUMN FROM LEVEL BELOW AND CONTINUING ON TO LEVEL ABOVE.

INDICATES STEEL COLUMN DISCONTINUING AT FLOOR LEVEL.

INDICATES WOOD STUD BUILT-UP COLUMN - SEE 2/S4.01 FOR TYPICAL

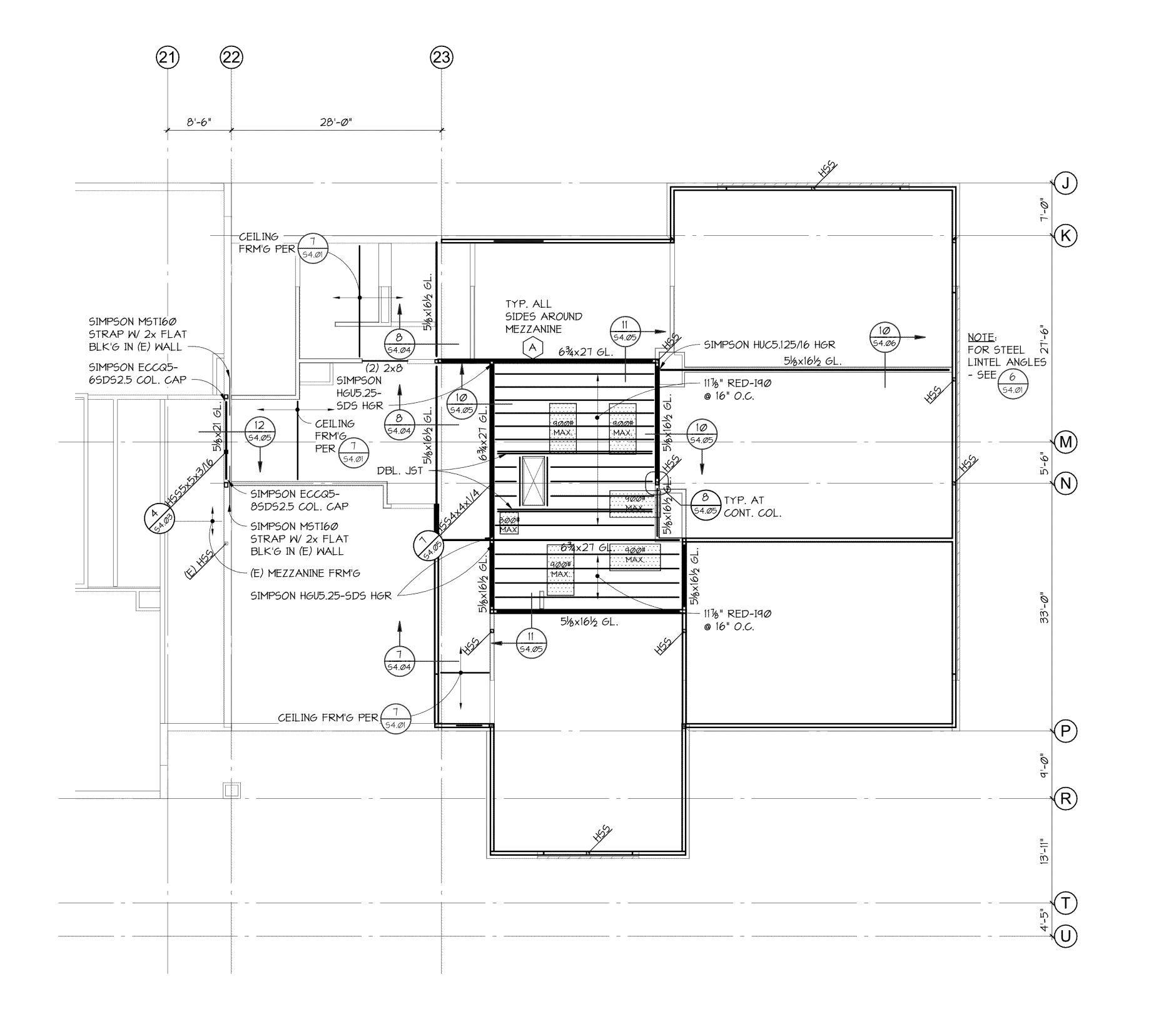
INDICATES SPECIAL WOOD STUD WALL TYPE - SEE 4/S4.01 FOR SCHEDULE. SEE S2.01 FOR WALLS THAT EXTEND FROM THE FOUNDATION TO THE ROOF.

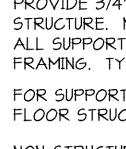
2 ATTIC FRAMING PLANS - NORTH PLAN

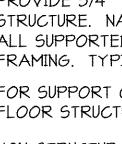
INDICATES HOLDOWN - SEE 1/S4.03 FOR SCHEDULE.

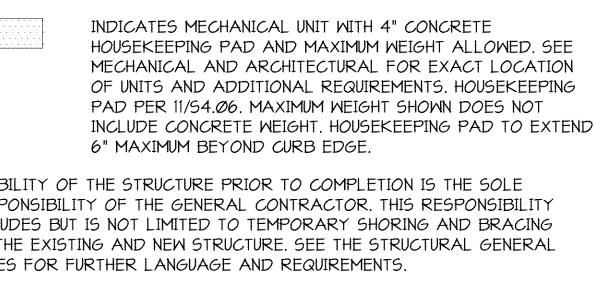
INDICATES PENETRATION IN FLOOR STRUCTURE.

- 12. PROVIDE 3/4" TONGUE AND GROOVE WOOD SHEATHING OVER ENTIRE FLOOR STRUCTURE. NAIL WOOD FLOOR SHEATHING WITH 8d AT 6" ON CENTER AT ALL SUPPORTED PANEL EDGES AND 8d AT 10" ON CENTER AT INTERMEDIATE FRAMING. TYPICAL UNLESS NOTED OTHERWISE - SEE 3/S4.01.
- 13. FOR SUPPORT OF MISCELLANEOUS MECHANICAL EQUIPMENT AND PIPES FROM FLOOR STRUCTURE - SEE SHEET S4.06.
- 14. NON-STRUCTURAL STUD WALLS ARE NOT SHOWN OR SHOWN SCREENED. FOR LOCATION - SEE ARCHITECTURAL DRAWINGS. FOR BRACING AT TOPS OF WALLS - SEE SHEET S4.06.
- INDICATES MECHANICAL UNIT WITH 4" CONCRETE HOUSEKEEPING PAD AND MAXIMUM WEIGHT ALLOWED. SEE MECHANICAL AND ARCHITECTURAL FOR EXACT LOCATION OF UNITS AND ADDITIONAL REQUIREMENTS. HOUSEKEEPING PAD PER 11/S4.06. MAXIMUM WEIGHT SHOWN DOES NOT INCLUDE CONCRETE WEIGHT. HOUSEKEEPING PAD TO EXTEND 6" MAXIMUM BEYOND CURB EDGE.
- 16. STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THIS RESPONSIBILITY INCLUDES BUT IS NOT LIMITED TO TEMPORARY SHORING AND BRACING OF THE EXISTING AND NEW STRUCTURE. SEE THE STRUCTURAL GENERAL NOTES FOR FURTHER LANGUAGE AND REQUIREMENTS.









architects

Jurisdiction Stamp Area

71 Columbia Street, Suite 500 Seattle, Washington 98104

T (206) 340 9500 F (206) 340 9519

CIVIL ENGINEER AND LANDSCAPE MacKay Sposito 1325 SE Tech Center Drive, Suite 140 Vancouver, WA 98683 T (360) 695 3411

STRUCTURAL ENGINEER PCS Structural Solutions One Main Place 101 SW Main Street, Suite 280 Portland, OR 97204 T (503) 232 3746

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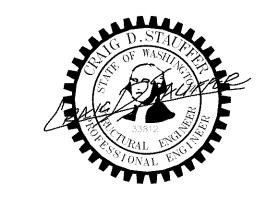
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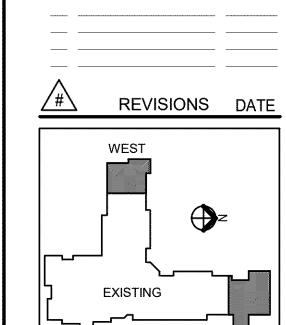
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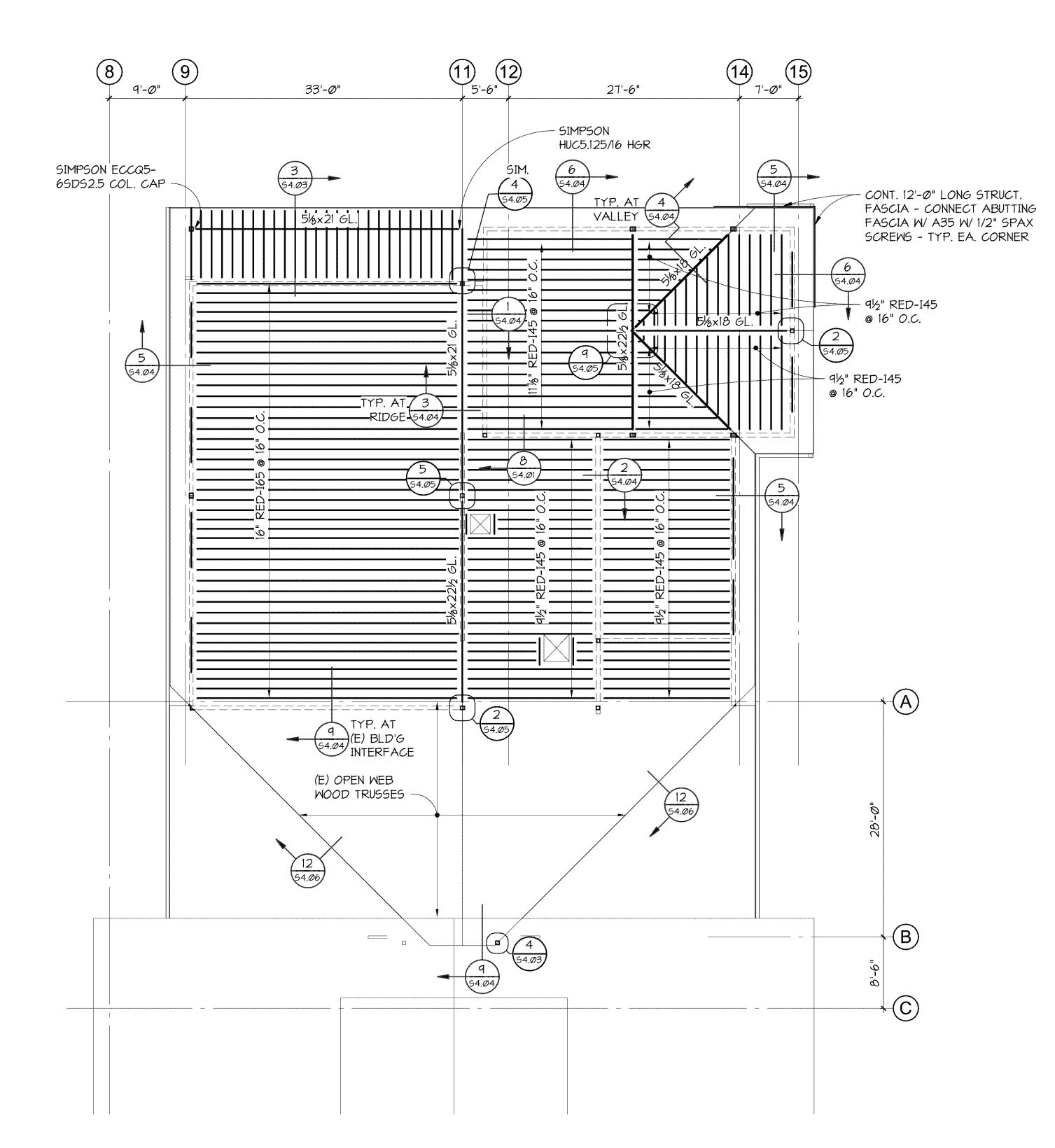
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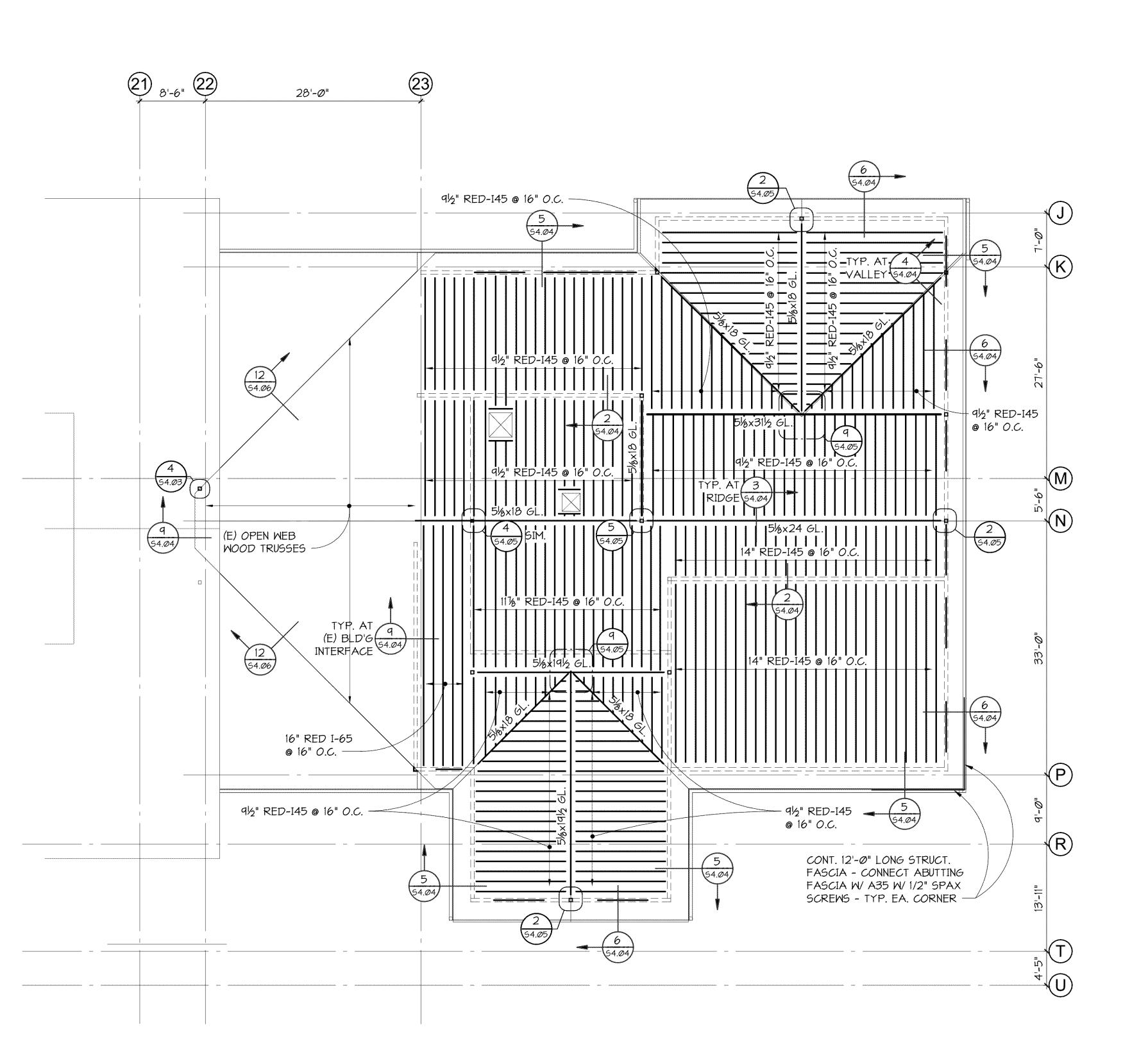
ATTIC FRAMING **PLANS** 

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#### ROOF FRAMING NOTES

- 1. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- 2. = INDICATES WALL EXTENDING TO ROOF STRUCTURE.
- INDICATES TYPICAL HEADER IN WALL BELOW SEE 1/S4.01.
- INDICATES PENETRATION IN ROOF STRUCTURE.
- INDICATES CAMBER FOR GLULAM BEAMS. C=0" UNLESS NOTED OTHERWISE.
- 6. PROVIDE 5/8" WOOD SHEATHING OVER ENTIRE ROOF STRUCTURE. NAIL SHEATHING PER 3/S4.01 TYPICAL UNLESS NOTED OTHERWISE.
- 7. FOR SUPPORT OF MISCELLANEOUS MECHANICAL EQUIPMENT AND PIPES FROM ROOF STRUCTURE - SEE SHEET S4.06.
- 8. NON-STRUCTURAL STUD WALLS ARE NOT SHOWN OR SHOWN SCREENED. FOR LOCATION - SEE ARCHITECTURAL DRAWINGS. FOR BRACING AT TOPS OF WALLS - SEE SHEET S4.06.
- 9. STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THIS RESPONSIBILITY INCLUDES BUT IS NOT LIMITED TO TEMPORARY SHORING AND BRACING OF THE EXISTING AND NEW STRUCTURE. SEE THE STRUCTURAL GENERAL NOTES FOR FURTHER LANGUAGE AND REQUIREMENTS.









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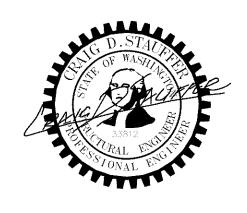
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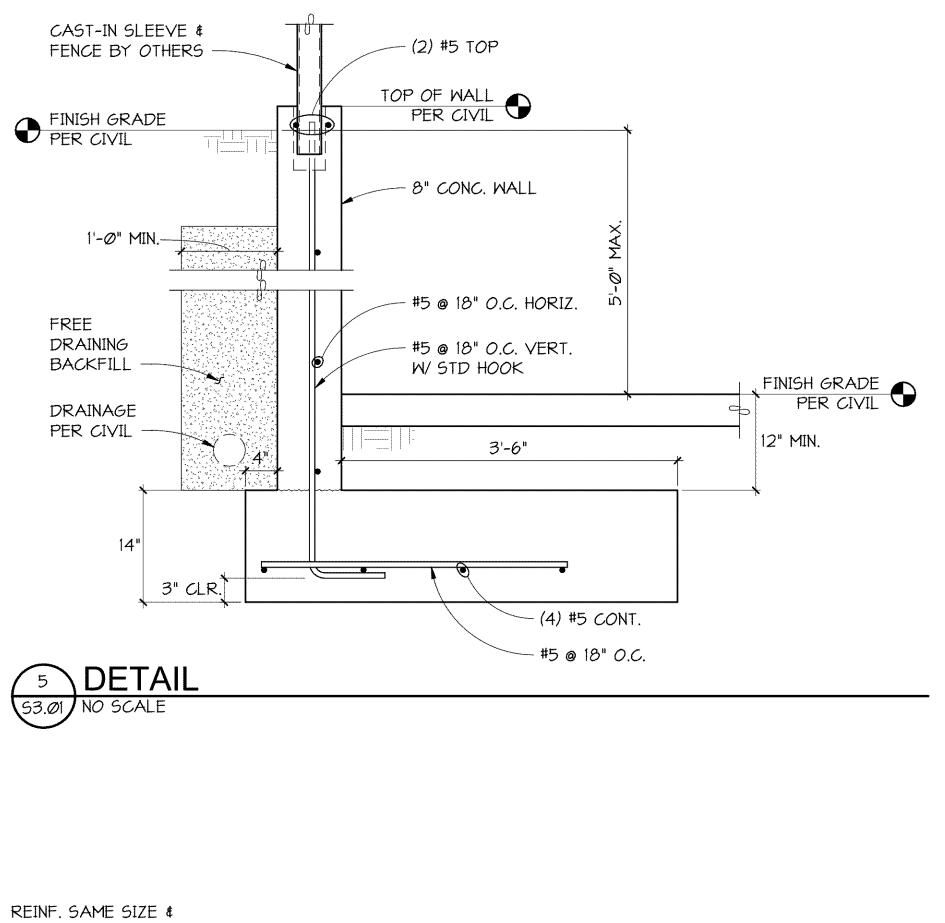
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**ROOF FRAMING** PLANS

S2.03



- 1/8" SAWCUTx1/4 SLAB DEPTH

- TOOLED JT R=1/8"

2'-0"

TYPICAL CONTROL JOINT FOR FIBER REINFORCED SLAB ON GRADE

1'-0"

TYPICAL CONSTRUCTION JOINT FOR FIBER REINFORCED SLAB ON GRADE

- SAW WITHIN 4 TO 12 HOURS OF

POURING SLAB - FILL W/ MASTIC

LOCATE JOINT'S AT NON-BEARING

WALLS WHERE POSSIBLE - SUBMIT PATTERN TO ARCHITECT FOR

2'-0"

APPROVAL.

CONT. 1x2

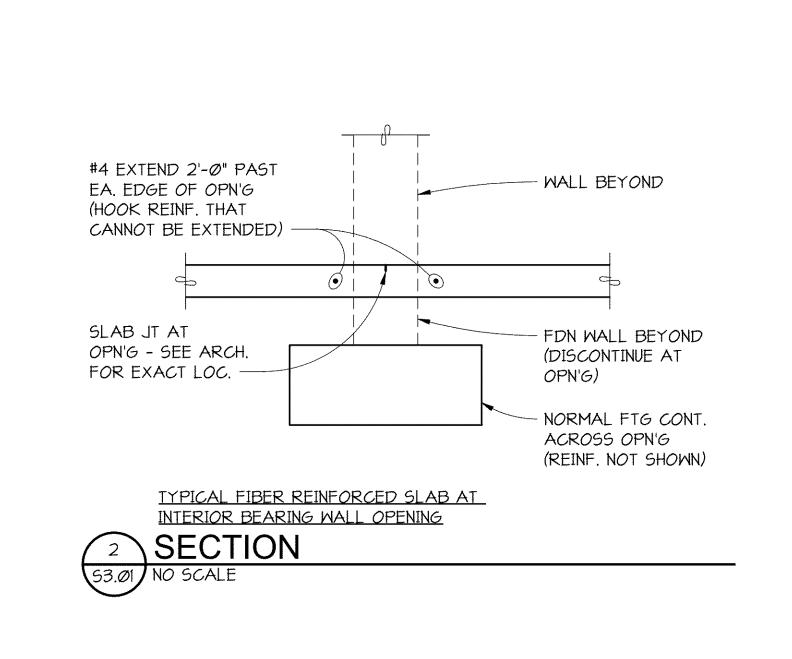
AT MID-DEPTH

SECTION

53.01 NO SCALE

DETAIL

NOMINAL SHEAR KEY



NOTE:
PROVIDE 2x INFILL FRAMING AT
EXISTING OPENINGS TO MATCH
EXISTING FRAMING AT 16" ON
CENTER. PROVIDE SHEATHING
AND NAILING PER 4 SCHEDULE.

- EDGE NAILING

- CONC. INFILL

(E) SHEAR WALL
- RE-NAIL & 4
ATTACH PER (54.01)

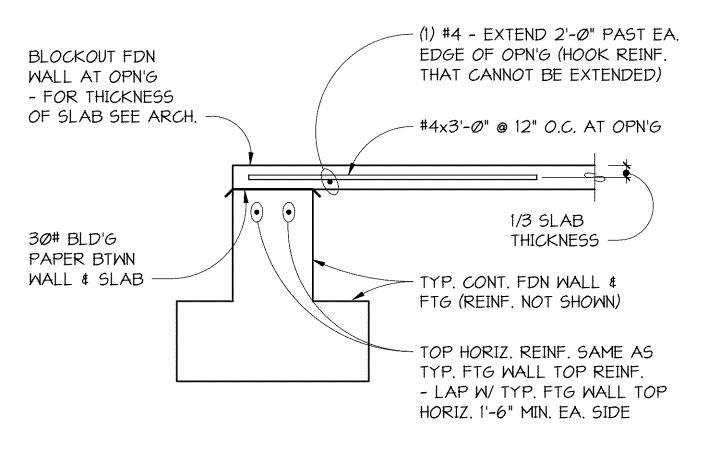
(E) 5.0.G. -

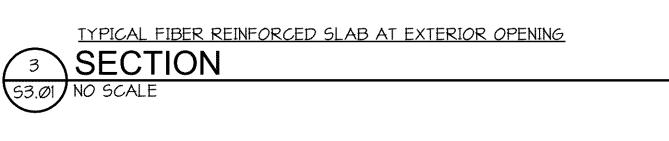
5/8"Ф

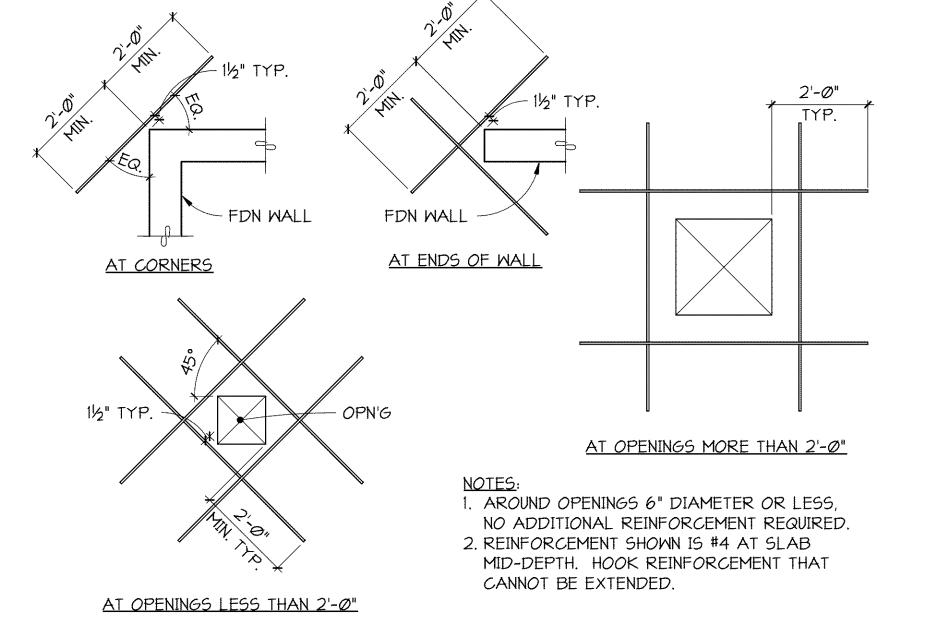
ADHESIVE ANCHORS

(E) FDN —

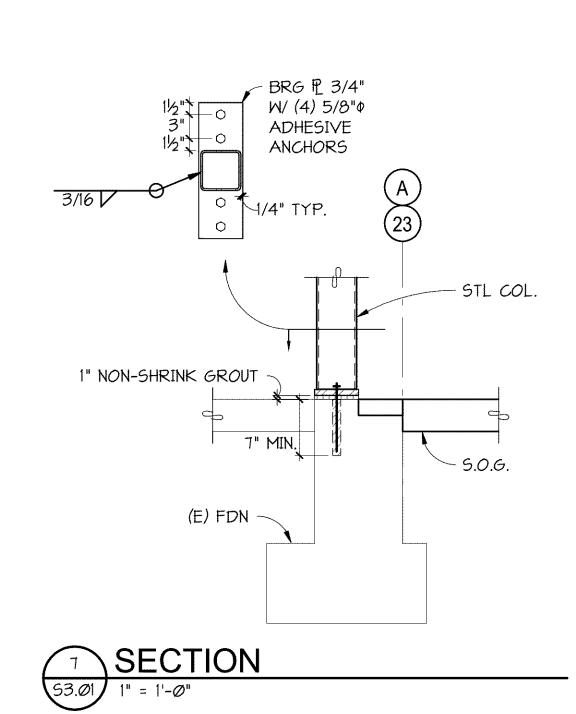
6 SECTION 53.01 1" = 1'-0"

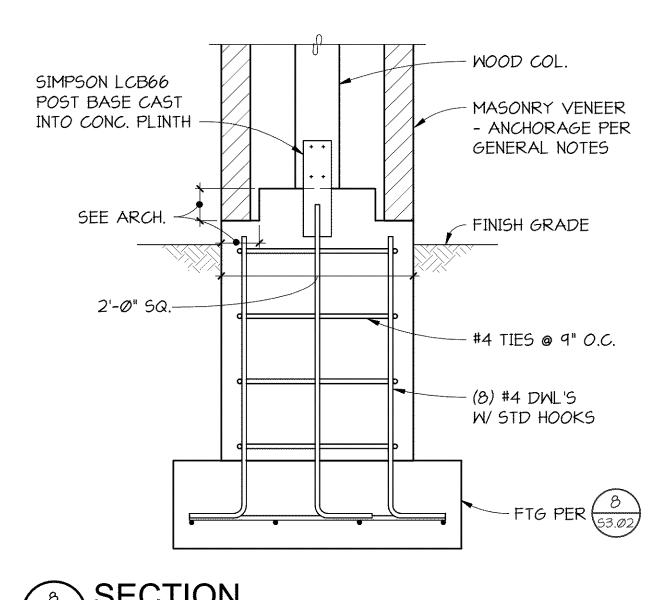


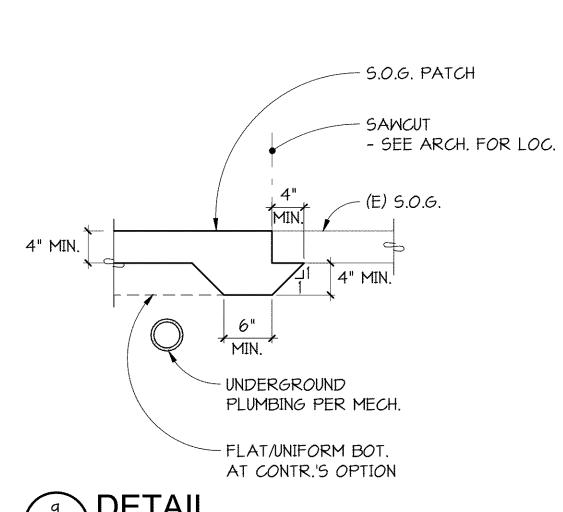












Jurisdiction Stamp Area

**Dassetti** architects

71 Columbia Street, Suite 500 Seattle, Washington 98104

T (206) 340 9500 F (206) 340 9519

CIVIL ENGINEER AND LANDSCAPE

1325 SE Tech Center Drive, Suite 140

MacKay Sposito

One Main Place

BCE Engineers

Fife, WA 98424

T (253) 922-0446

BCE Engineers

Fife, WA 98424 T (253) 922-0446

ACOUSTIC

Portland, OR 97204 T (503) 232 3746

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STRUCTURAL ENGINEER PCS Structural Solutions

MECHANICAL ENGINEER

6021 12th St E, Suite 200

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Lynnwood, WA 98036

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AT FULL SIZE

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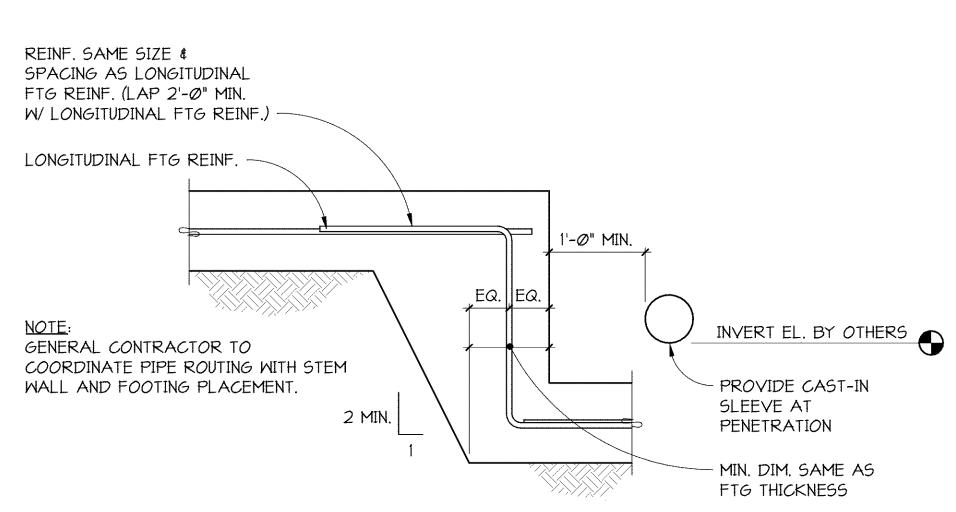
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Portland, OR 97209

T (503) 226 2730

101 SW Main Street, Suite 280

9 **DETAIL** 53.01 1" = 1'-0"



8 SECTION 53.01 1" = 1'-0"

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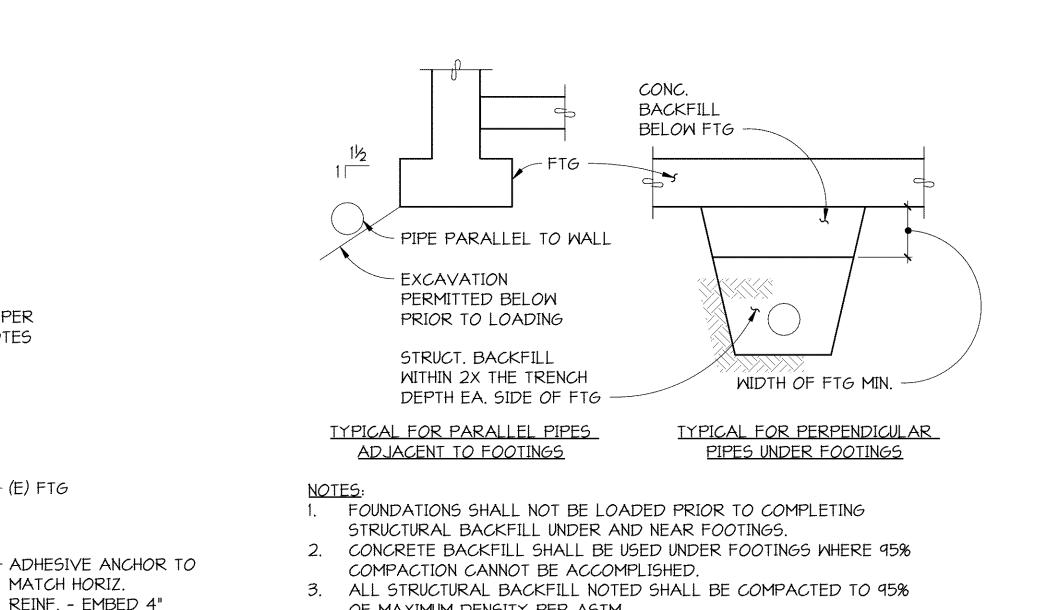
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**FOUNDATION DETAILS** 

S3.01



STEM WALL NEW FOOTING/EXISTING FOOTING CONNECTION

- LAP SPLICE PER

GENERAL NOTES

MATCH HORIZ.

CONC. FTG &

REINF. - EMBED 4"

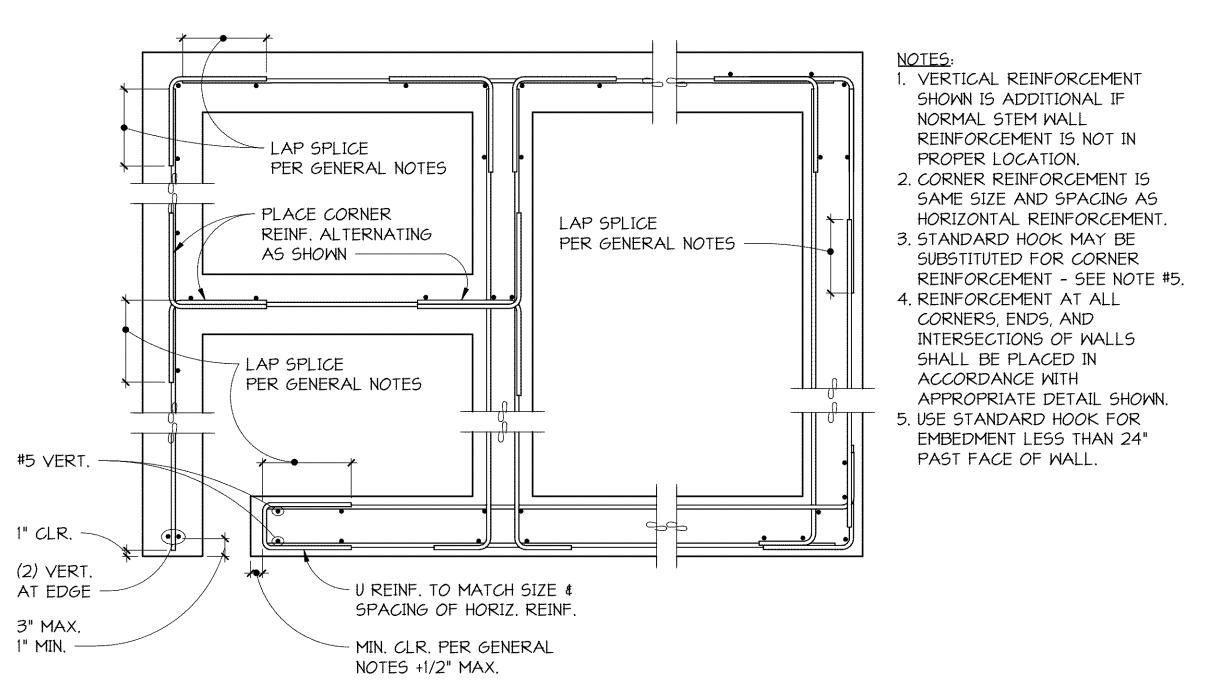
SECTION 53.02 NO SCALE

OF MAXIMUM DENSITY PER ASTM. 4. A PIPE SLEEVE SHALL BE PROVIDED FOR SHALLOW PIPES CAST IN CONCRETE.

5. PIPES SHALL NOT BE PLACED IN THE FOOTING WITHOUT SPECIFIC APPROVAL FROM THE ENGINEER.

6. FOR VARIATIONS CONTACT ENGINEER.

2 SECTION 53.02 NO SCALE



TYPICAL REINFORCEMENT PLACEMENT FOR FOUNDATIONS AND STEM WALLS

3 DETAIL 53.02 NO SCALE

SEE ARCH. FOR REVEALS - 1½" DEEP x 1/3 WALL WIDTH KEY WAY - LAP SPLICES PER GENERAL 2½" MIN. EITHER SIDE **★**•••• 1/2" WIDTH OF

TYPICAL AT VERTICAL WALL JOINTS

OBTAIN APPROVAL OF ENGINEER FOR LOCATION OF ANY CONSTRUCTION JOINT

WATERSTOP MIN.

MARK

F3.Ø

F5.Ø

<u>NOTES</u>

3'-Ø"

5'-Ø"

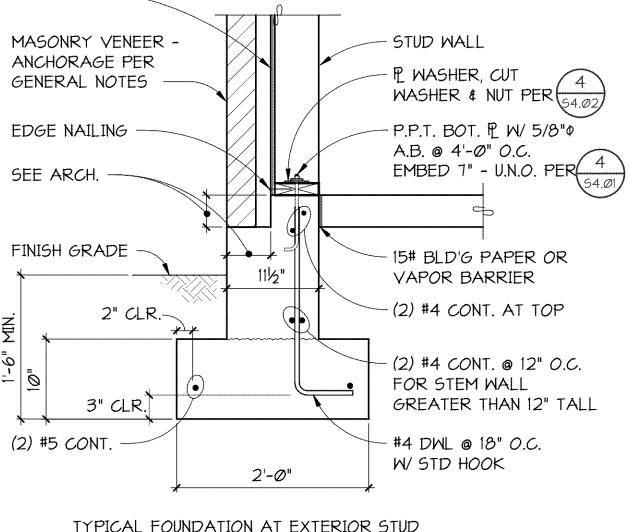
**SECTION** 

53.02 NO SCALE

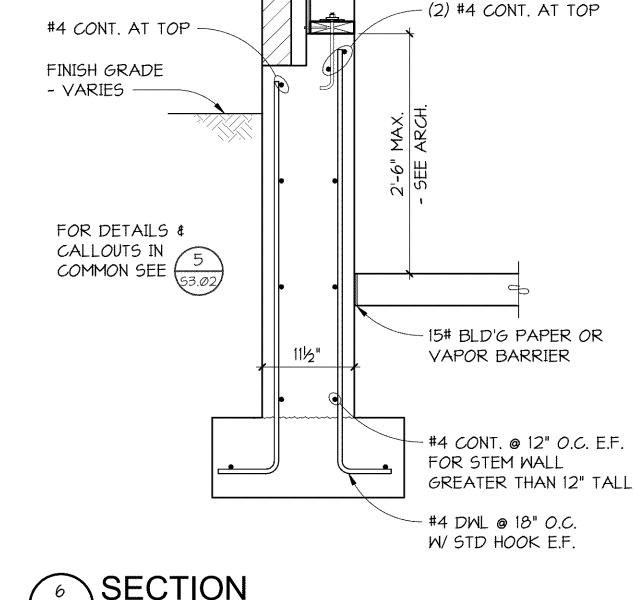
- LAP SPLICES PER # GENERAL NOTES ROUGH INTERFACE W/ 1/4" UNIFORM AMPLITUDE NORMAL REINF. CONT. THROUGH JT PER APPLICABLE DETAILS

TYPICAL AT CONCRETE FOOTINGS

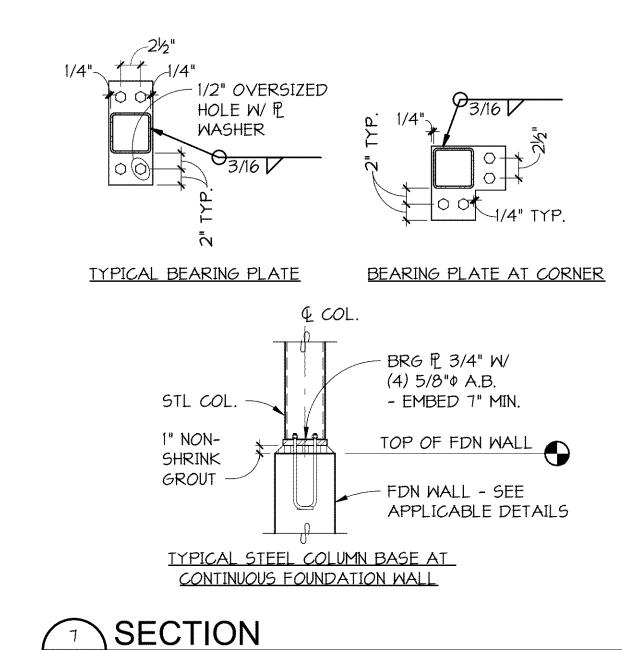
5 SECTION 53.02 NO SCALE



TYPICAL FOUNDATION AT EXTERIOR STUD WALL WITH MASONRY VENEER



6 SECTION 53.02 NO SCALE



S3.02 NO SCALE

4 CONSTRUCTION JOINTS 53.02 NO SCALE

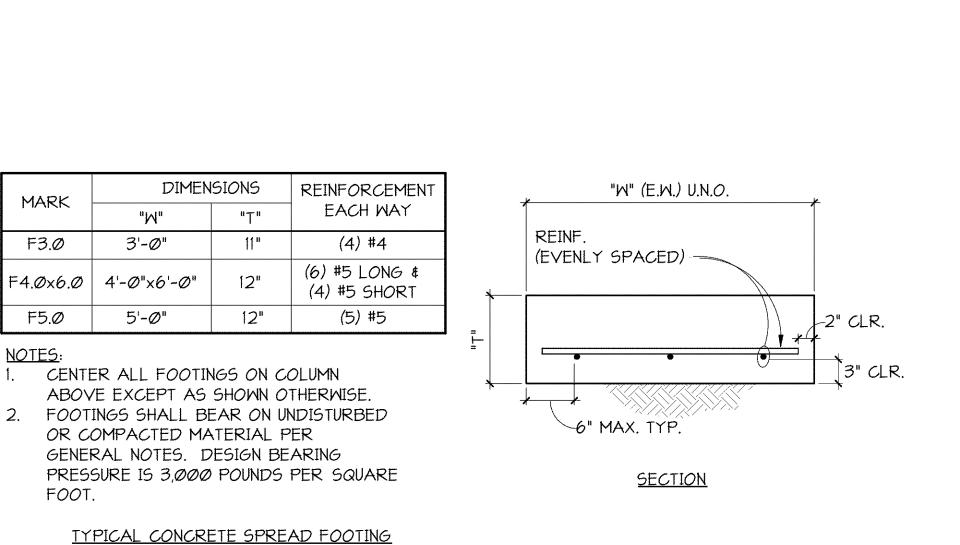
TYPICAL FOR CAST IN PLACE FOUNDATIONS AND STEM WALLS

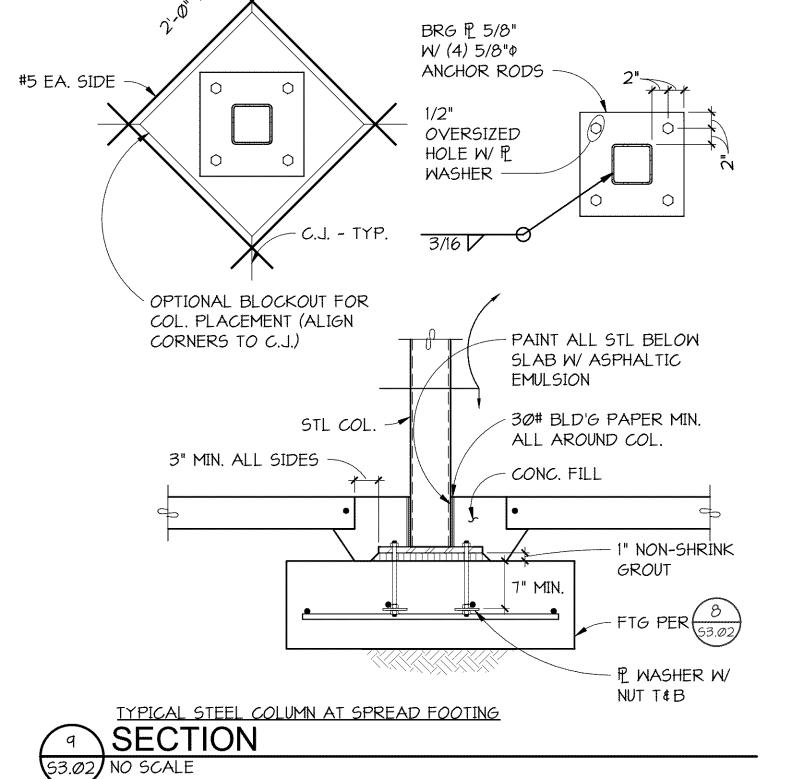
6" PVC CONT.

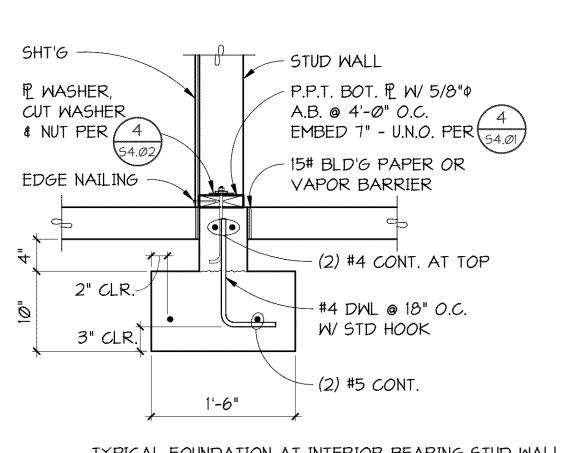
WATERSTOP IF EXT.

WALL OR BASEMENT

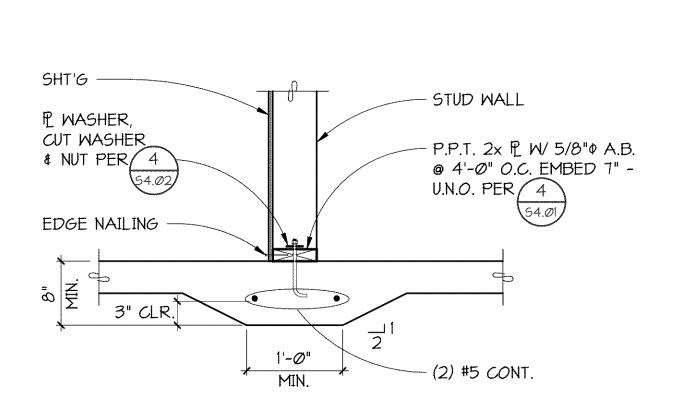
(M/ WELDED JT)







TYPICAL FOUNDATION AT INTERIOR BEARING STUD WALL SECTION SECTION 53.02 NO SCALE



TYPICAL THICKENED SLAB FOR NON-BEARING INTERIOR SHEAR WALL

" SECTION 53.02

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71 Columbia Street, Suite 500 Seattle, Washington 98104 T (206) 340 9500 F (206) 340 9519

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STRUCTURAL ENGINEER PCS Structural Solutions One Main Place 101 SW Main Street, Suite 280 Portland, OR 97204 T (503) 232 3746

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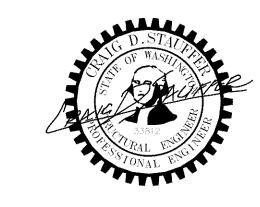
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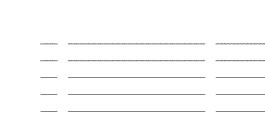
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**FOUNDATION DETAILS** 

S3.02

BEAM AT HIP BEAM AT WALL END BEAM AT WALL (6) 16d EA. SIDE - TYP. A35 -STUD SIDE PW. FILLER - 3/4" MAX.

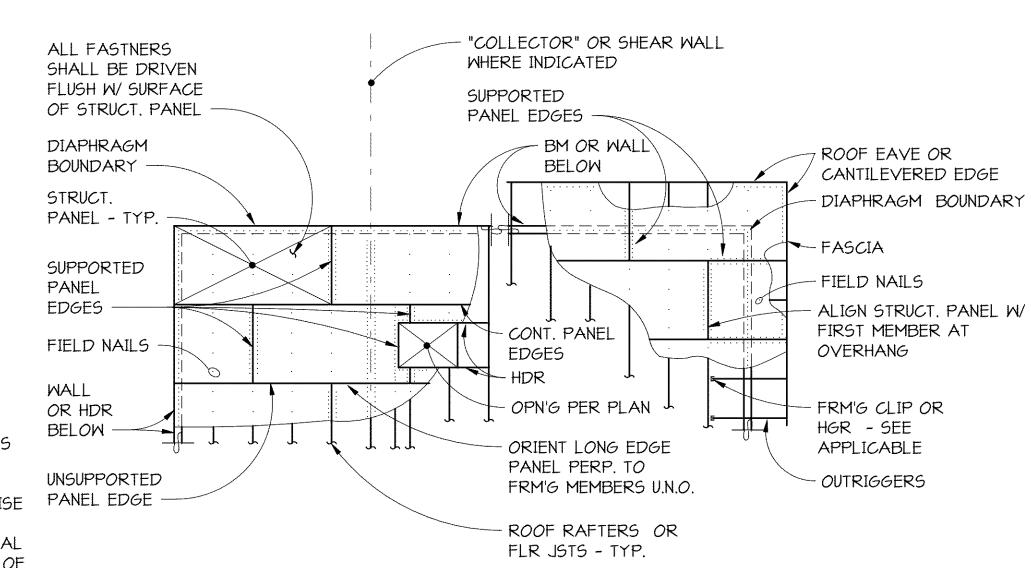
S4.01 NO SCALE

TYPICAL BUILT-UP COLUMN AT BEAM PERPENDICULAR TO WALL 2 DETAIL

DIAPHRAGM NAILING SCHEDULE DIAPHRAGM LOCATION NAILS SPACING TYPE FLOOR DIAPHRAGM 10d 6" 0.0 DIAPHRAGM BOUNDARY 23/32" TONGUE AND GROOVE FIELD NAILS 10d 12" O.C. SHEATHING UNBLOCKED UNLESS NOTED SUPPORTED 10d 6" 0.C PANEL EDGES OTHERWISE DIAPHRAGM ROOF 10d 6" O.C. DIAPHRAGM BOUNDARY 19/32" SHEATHING FIELD NAILS 10d 10" O.C. UNBLOCKED UNLESS NOTED SUPPORTED 10d 6" 0.C OTHERWISE PANEL EDGES

1. PROVIDE (2) ROWS OF SPECIFIED DIAPHRAGM BOUNDARY NAILING OVER INTERIOR SHEAR WALLS AND THE FULL LENGTH OF "COLLECTORS" WHERE INDICATED.

2. AT BLOCKED DIAPHRAGMS PROVIDE 2x4 FLATWISE PANEL EDGE BLOCKING WITH "Z2" CLIPS AT EACH END AT ALL UNSUPPORTED PANEL EDGES. USE 2x4 STRUCTURAL COMPOSITE LUMBER FLATWISE BLOCKING IN LIEU OF SOLID SAWN WHERE NAILING SIZE OR SPACING EXCEEDS 10d AT 4" ON CENTER.



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architects

71 Columbia Street, Suite 500

T (206) 340 9500 F (206) 340 9519

CIVIL ENGINEER AND LANDSCAPE

1325 SE Tech Center Drive, Suite 140

Seattle, Washington 98104

MacKay Sposito

T (360) 695 3411

One Main Place

Portland, OR 97204

T (503) 232 3746

BCE Engineers

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T (253) 922-0446

T (253) 922-0446

Vancouver, WA 98683

STRUCTURAL ENGINEER

101 SW Main Street, Suite 280

PCS Structural Solutions

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6021 12th St E, Suite 200

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Brewery Block 2

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TYPICAL STUD WALL CONSTRUCTION AT HEADER

SECTION 54.01 NO SCALE

TYPICAL DIAPHRAGM NAILING SCHEDULE 54.01 1" = 1'-0"

	STUD WALL CONSTRUCTION SCHEDULE									
				TAE	BLE 1 - SHE	AR WALL RE	QUIREMENT	TS		
MARK	WALL SHEATHING	IAITTII	SHEATHING NAILS NOTE 2	EDGE NAILING ON CENTER	EDGE FRAMING NOTE 5	FIELD NAILING ON CENTER	BOTTOM PLATE NOTE 6	BOTTOM PLATE NAILING	5/8" ANCHOR BOLT SPACING (EMBED 7" MINIMUM)	RIM/BLOCKING CONNECTOR TO TOP PLATE BELOW
$\langle A \rangle$	15/32"	(1)	8d	6"	2x	12"	2x	16d @ 8" O.C.	48"	A35 @ 24" O.C.
⟨B⟩	15/32"	(1)	8d	4"	2x	12"	2x	(2) 16d @ 8" O.C.	32"	A35 @ 16" O.C.
										<u>I</u>

REQUIREMENTS PER TABLE 1

TABLE 2 - STUD REQUIREMENTS					
MARK	STUD SIZE AND SPACING	NOTES	No		
1	3½"x5½" SCL @ 16" O.C.	MHERE WALL HT ≥ 12'-Ø"	1.		
			I		

- SECOND CHARACTER INDICATES SPECIAL STUD SPACING PER TABLE 2 INDICATES SPECIAL STRUCTURAL WALL MARK. ALL WALLS SHOWN ON STRUCTURAL

FIRST CHARACTER INDICATES SPECIAL SHEAR WALL

DRAWINGS ARE 2x6 AT 16" ON CENTER UNLESS DESIGNATED SPECIAL. STUD LAYOUT SHALL MATCH FRAMING MEMBER LAYOUT ABOVE WHERE APPLICABLE. ALL EXTERIOR WALLS SHALL HAVE 15/32" WOOD SHEATHING AND BE NAILED WITH 10d AT 6" ON CENTER AT EDGES AND 12" ON CENTER IN FIELD UNLESS DESIGNATED SPECIAL.

2. ALL EXTERIOR WALLS AND ALL DESIGNATED SHEAR WALLS SHALL BE BLOCKED AT ALL SHEATHING EDGES. EDGE NAILING APPLIES TO ALL TOP AND BOTTOM PLATES, VERTICAL JOINTS, HORIZONTAL BLOCKED JOINTS, WALL CORNERS, AND HOLDOWN ANCHORED STUDS. 3. WHERE BEAMS OR HEADERS FRAME INTO WALLS AND A COLUMN IS NOT CALLED OUT, PROVIDE BUILT-UP COLUMNS PER 2/S4.ØI FOR BEAM

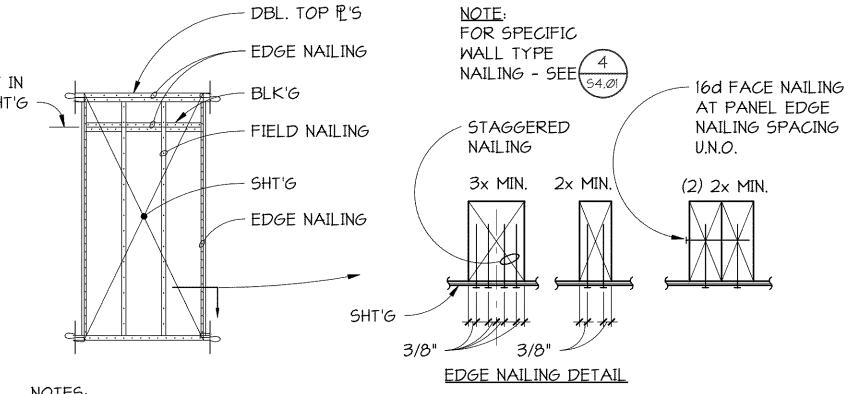
PERPENDICULAR TO WALL. 4. [X,Y] INDICATES BUILT-UP STUD COLUMNS AT HEADERS IN WALLS - SEE 1/S4.Ø1 FOR BEAM PARALLEL TO WALL.

5. PROVIDE 3x OR DOUBLE 2x MEMBERS FACE NAILED PER 5/S4.ØI AT ALL ABUTTING PANEL EDGES WHERE INDICATED.

6. 3x BOTTOM PLATE WHERE INDICATED.

7. WHERE SOLID SAWN STUD LENGTH CANNOT BE OBTAINED, STRUCTURAL COMPOSITE LUMBER STUDS MAY BE SUBSTITUTED. SOLID SAWN FRAMING MAY NOT BE SUBSTITUTED FOR SPECIFIED STRUCTURAL COMPOSITE LUMBER FRAMING.

4 SCHEDULE 54.01 NO SCALE



1. PANEL EDGE NAILING AND PLATE NAILING SHALL BE STAGGERED IN ALL CASES. 2. SHEATHING JOINT SHALL OCCUR AT COMMON MEMBER UNLESS IT OCCURS AT A

SPECIFIED DOUBLE MEMBER. 3. EDGE NAILING APPLIES TO AREAS INDICATED AND AT HOLDOWN ANCHORED STUDS.

TYPICAL SHEAR WALL NAILING 5 SCHEDULE S4.01 NO SCALE

LINTEL ANGLE SCHEDULE						
MAXIMUM WIDTH OF OPENING	ANGLE SIZE	MINIMUM BEARING REQUIRED AT END OF ANGLE				
5'-0"	L3½x3½x1/4	4 <sup>8</sup>				
8'-Ø"	L6x31/2x5/16	4"				
12'-0"	L7x4x3/8	8"				

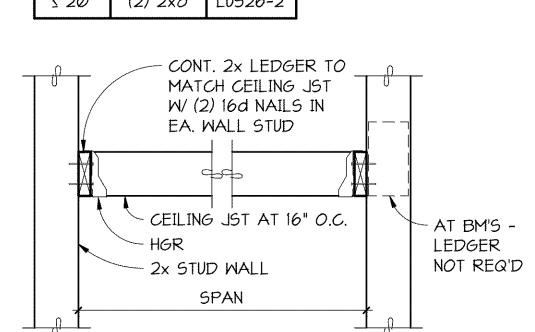
NOTE:

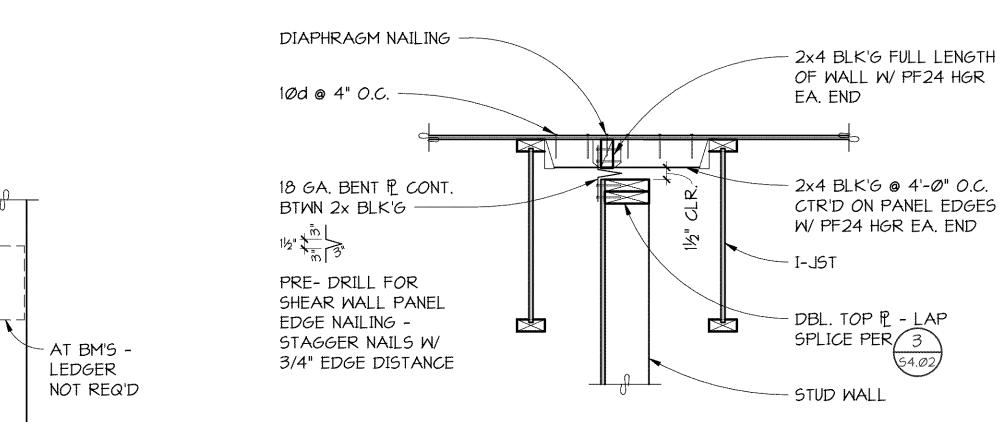
1. ALL LINTEL ANGLES SHALL BE GALVANIZED.

6 DETAIL S4.01 NO SCALE

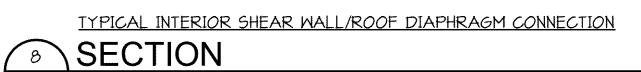
SPAN	CEILING JOIST	HANGER
≤ 10'	2×6	LUS26
₹ 12'	2x8	LUS26
<u> </u>	2x12	LUS210
≤ 16′	(2) 2x6	LUS26-2
≤ 20'	(2) 2×8	LUS26-2

7 **DETAIL** 54.01 1" = 1'-0"





54.01 1" = 1'-0"



Vancouver School District FRANKLIN **ELEMENTARY** SCHOOL **ADDITION** 

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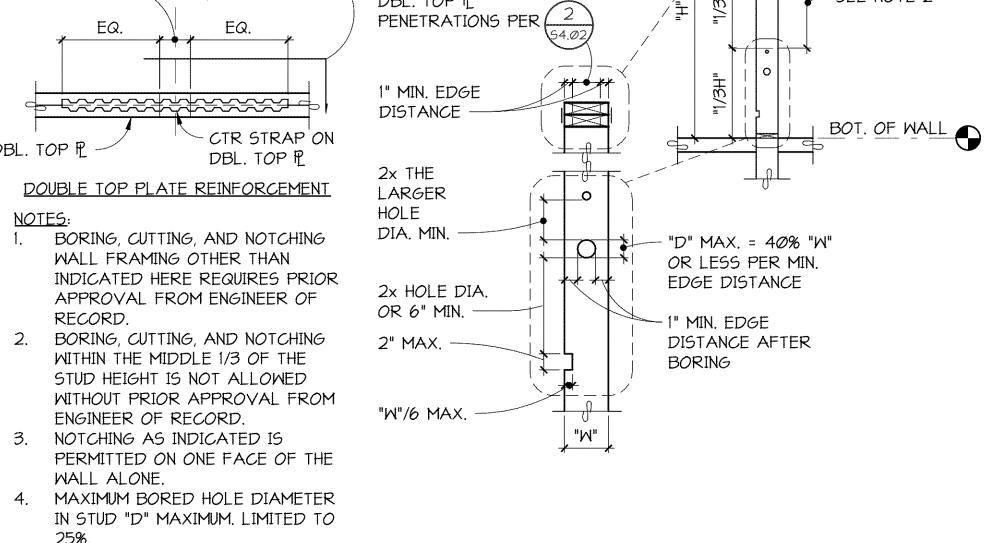
5206 NW Franklin St, Vancouver, WA 98663

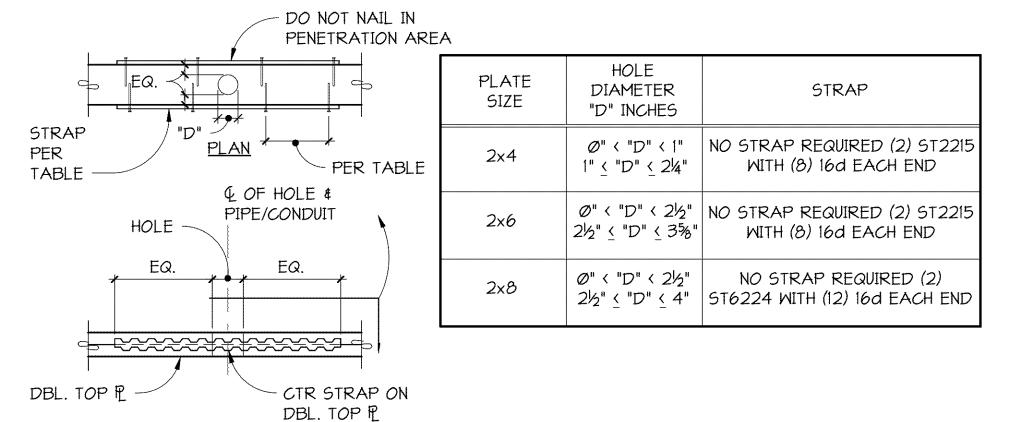
ISSUE DATE:

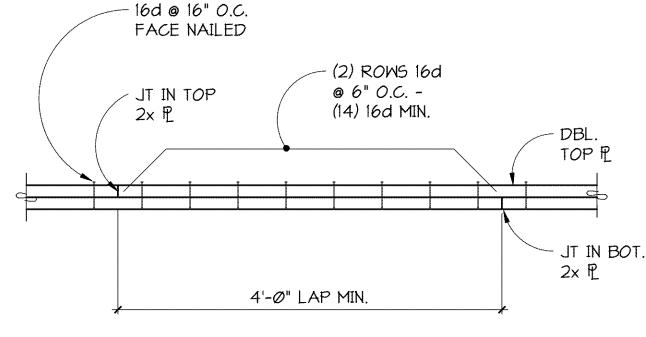
Stamp Area

WOOD FRAMING **DETAILS** 



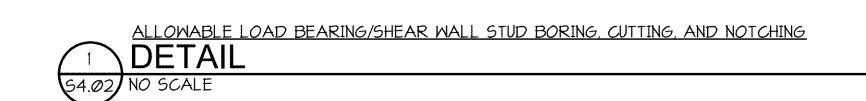








- EDGE NAILING



EQ. -

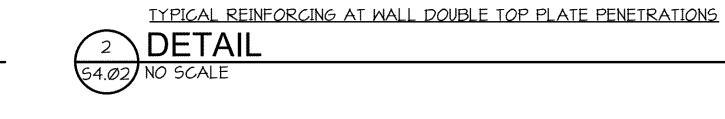
RECORD.

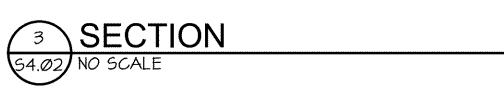
WALL ALONE.

"W" AT BUILT-UP STUDS, AND

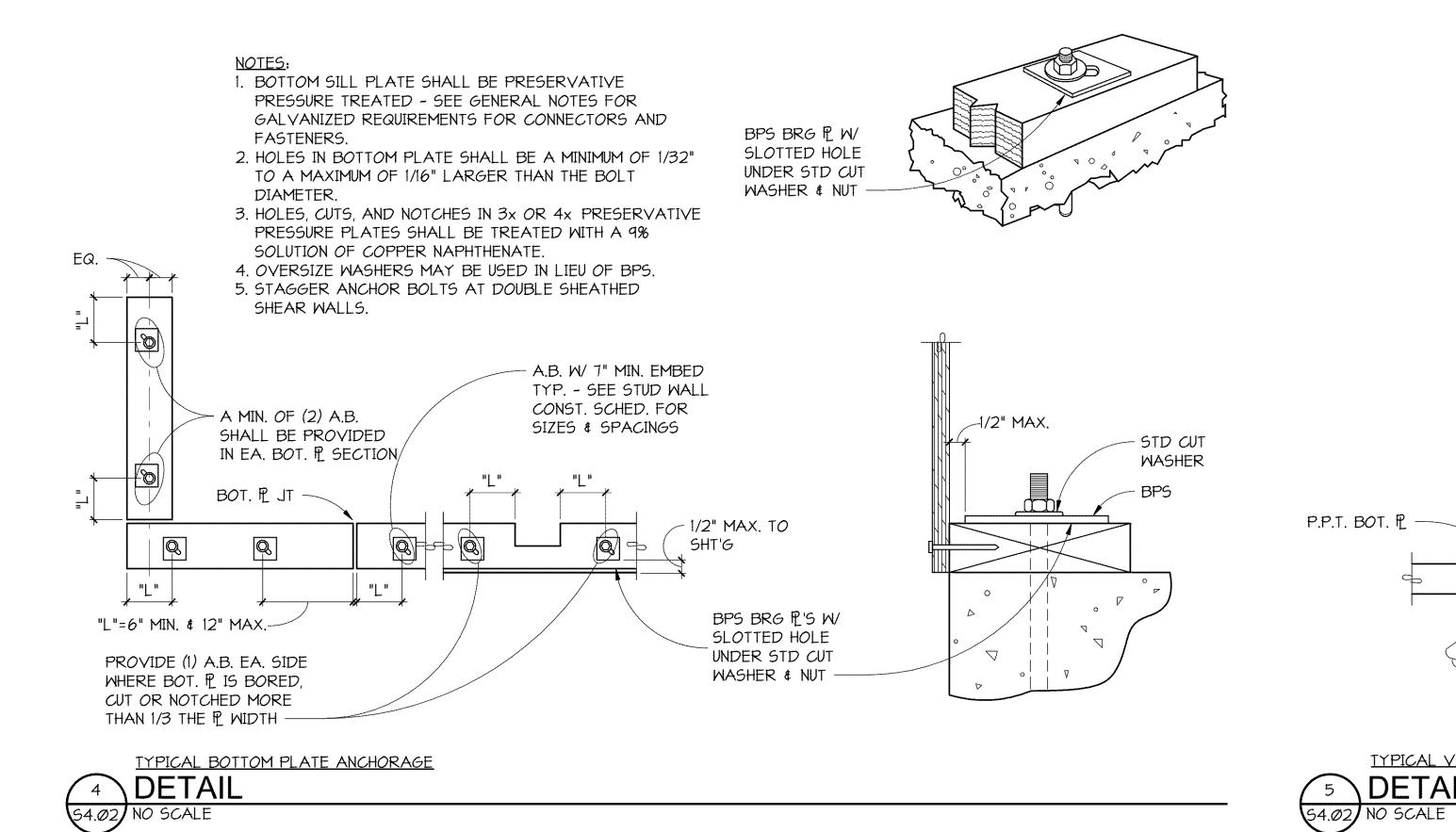
HOLDOWN ANCHORED STUDS.

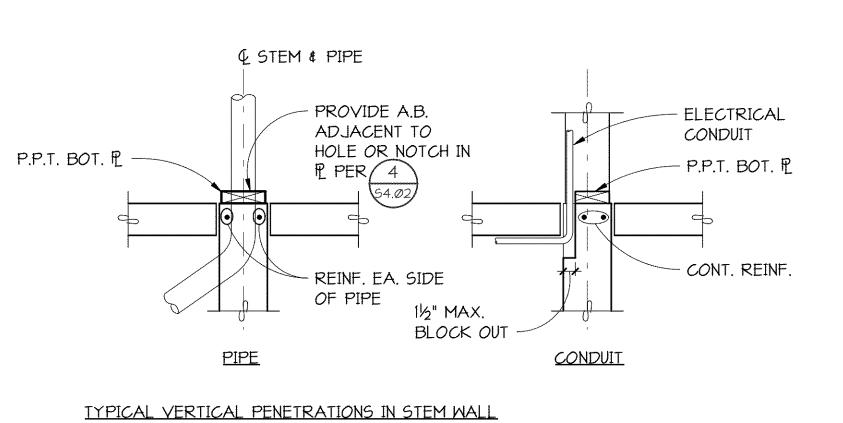
25%.



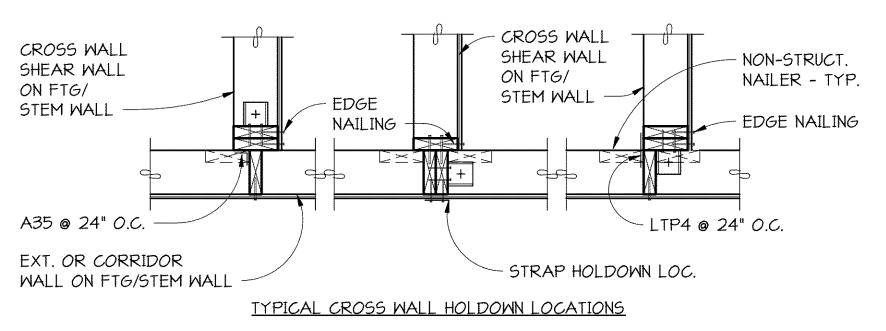


TYPICAL ANCHOR BOLT HOLDOWN AT CORNER





5 DETAIL



- TYP.

— A.B. HOLDOWN

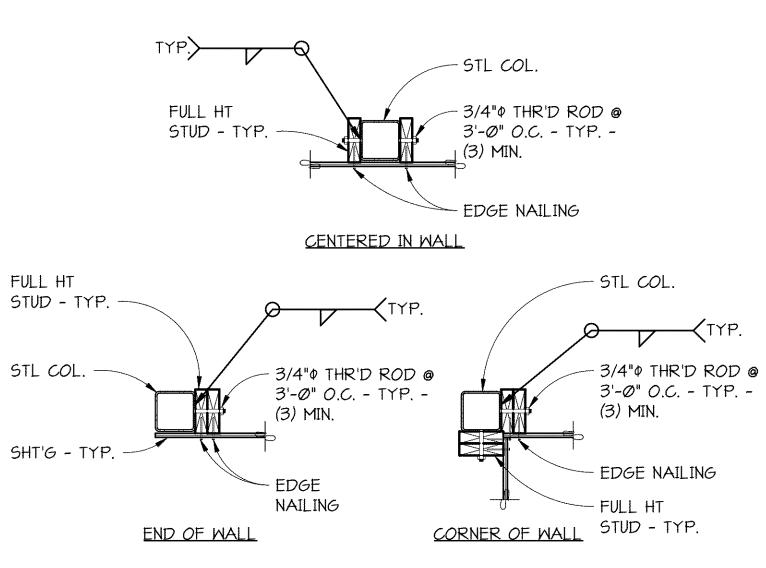
A.B. HOLDOWN

ALTERNATE HOLDOWN LOCATION AT CORNER



NON-STRUCT.

NAILER - TYP.





54.02 NO SCALE

architects 71 Columbia Street, Suite 500

Seattle, Washington 98104

T (206) 340 9500 F (206) 340 9519

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STRUCTURAL ENGINEER PCS Structural Solutions One Main Place 101 SW Main Street, Suite 280 Portland, OR 97204 T (503) 232 3746

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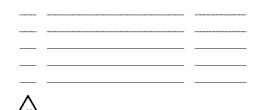
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COST CONSULTANT Rider Levett Bucknall (RLB) Brewery Block 2 1120 NW Couch Street, Suite 730 Portland, OR 97209 T (503) 226 2730

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Vancouver School District FRANKLIN **ELEMENTARY** SCHOOL ADDITION

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ISSUE DATE:

Stamp Area

WOOD FRAMING **DETAILS** 

Jurisdiction Stamp Area

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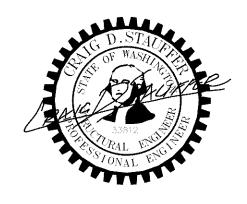
ELECTRICAL ENGINEER BCE Engineers 6021 12th St E, Suite 200 Fife, WA 98424 T (253) 922-0446

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REVISIONS DATE

Vancouver School District FRANKLIN **ELEMENTARY** SCHOOL

5206 NW Franklin St, Vancouver, WA 98663

**ADDITION** 

ISSUE DATE:

Stamp Area

WOOD FRAMING **DETAILS** 

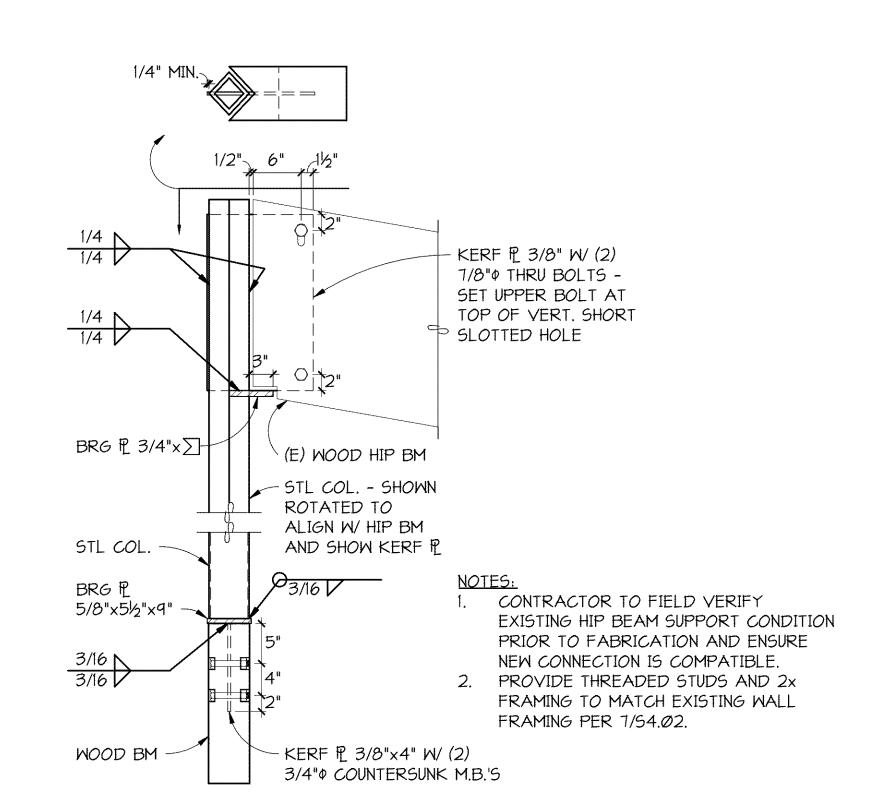
HOLDOWN SCHEDULE									
	FOUNDATION ANCHOR ROD TYPE 1								
MARK	HOLDOWN		NCHOR ROD <sup>2</sup> REINFORCEMENT	REQUIRED STUDS	REFERENCED DETAILS				
2	HDU2	5/8"	(2) #4	(2) 2x	2/54.03				
(4)	HDU4	5/8"	(2) #4	(2) 2x	2/54.03				
<b>5</b>	HDU5	5/8"	(2) #4	(2) 2x	2/54.03				
8	HDU8	7/8"	(2) #4	(3) 2x	2/54.03				
<u> </u>	HDU11	111	(2) #4	6x	2/54.03				
(14)	HDU14	Į H	(4) #4	6x	2/54.Ø3				

NOTES:

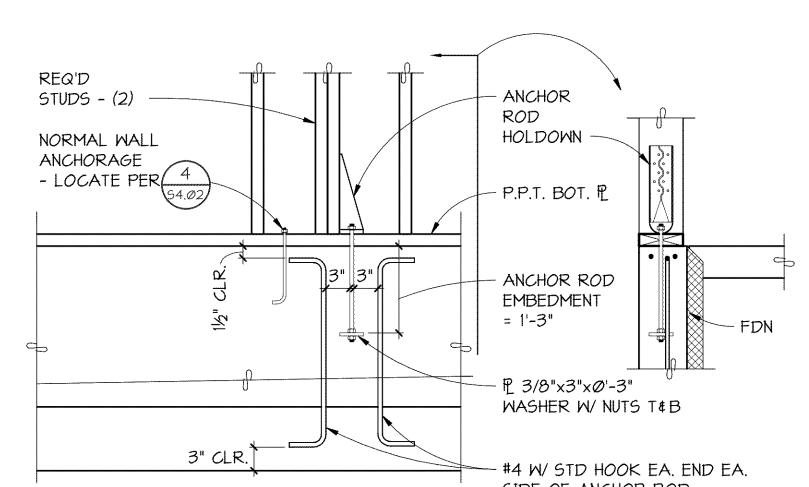
1. ALL HOLDOWNS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

1. TIBEAD BOD ACTM 436 WITH 3"x3"x3/8" PLATE WITH DOUBLE NUTS AT FOUN 2. ALL-THREAD ROD ASTM A36 WITH 3"x3"x3/8" PLATE WITH DOUBLE NUTS AT FOUNDATION.





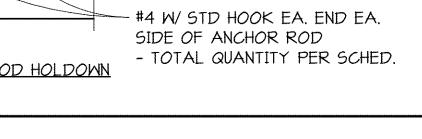


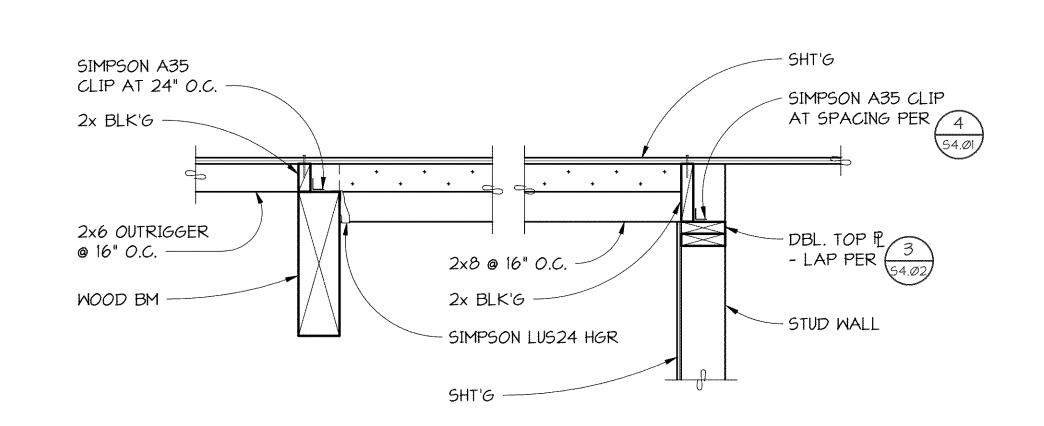


TYPICAL FOUNDATION ANCHOR ROD HOLDOWN

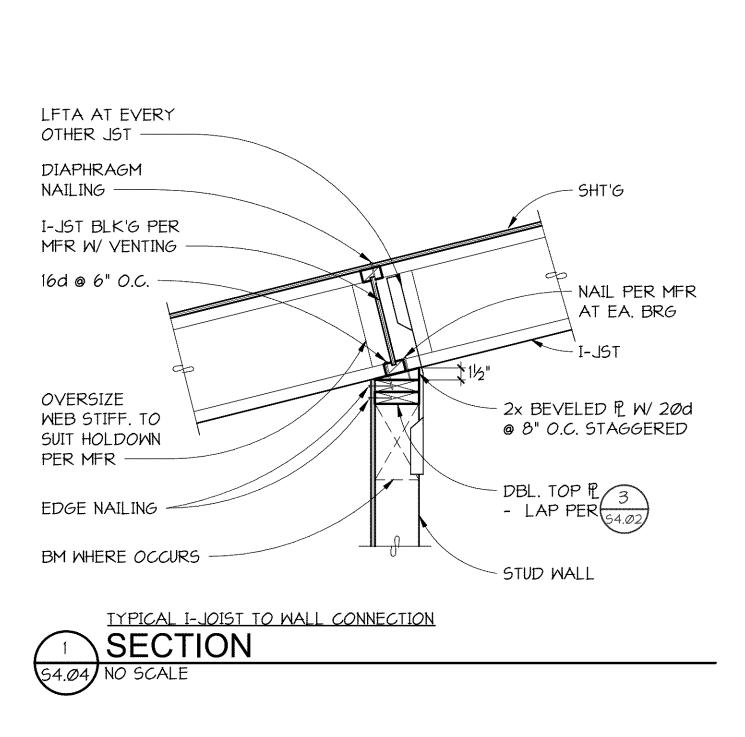
DETAIL

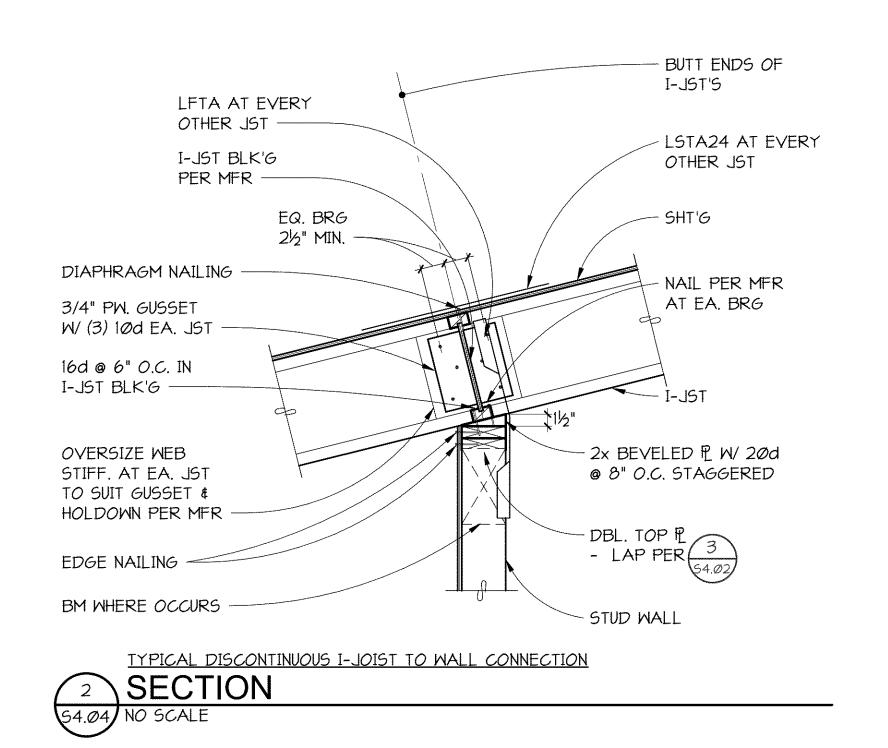
S4.03 NO SCALE

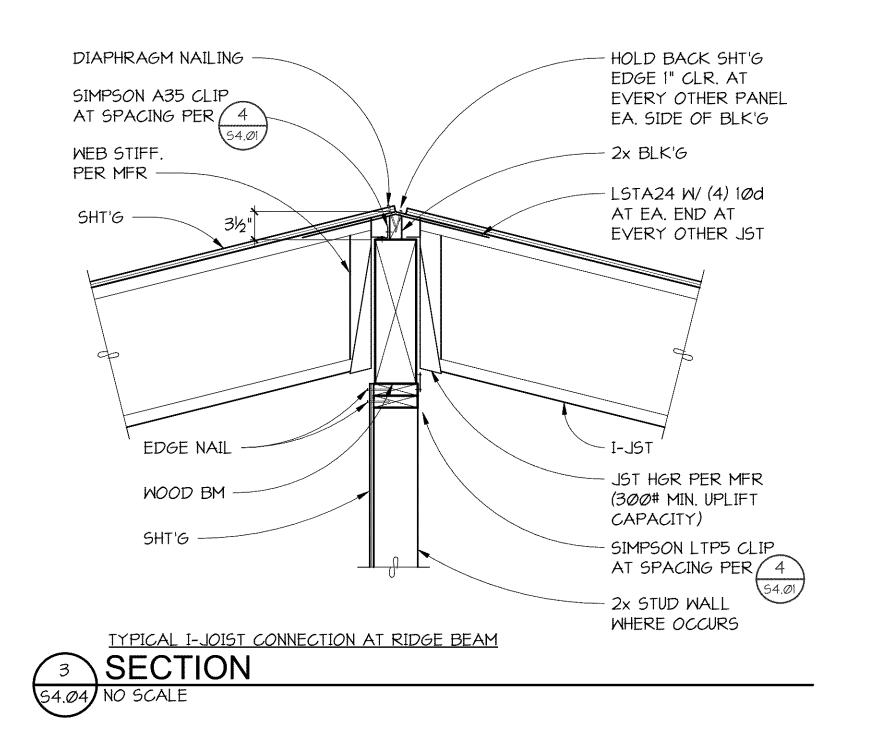


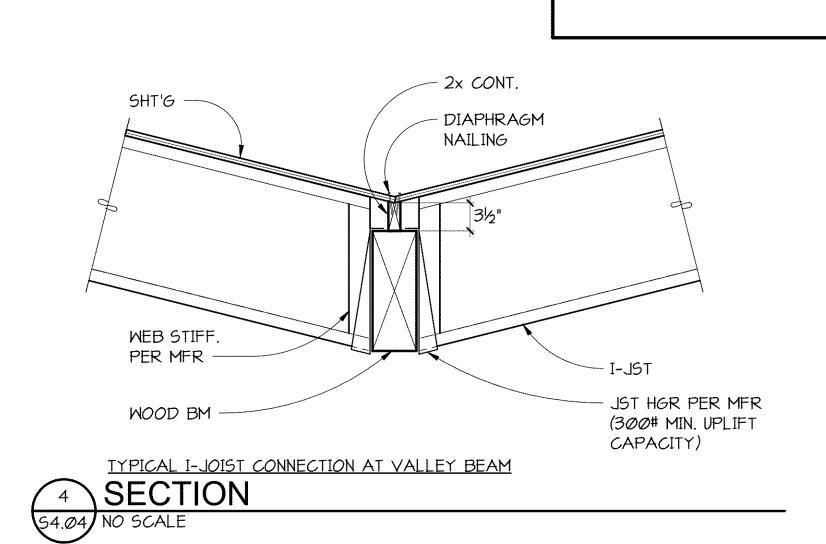


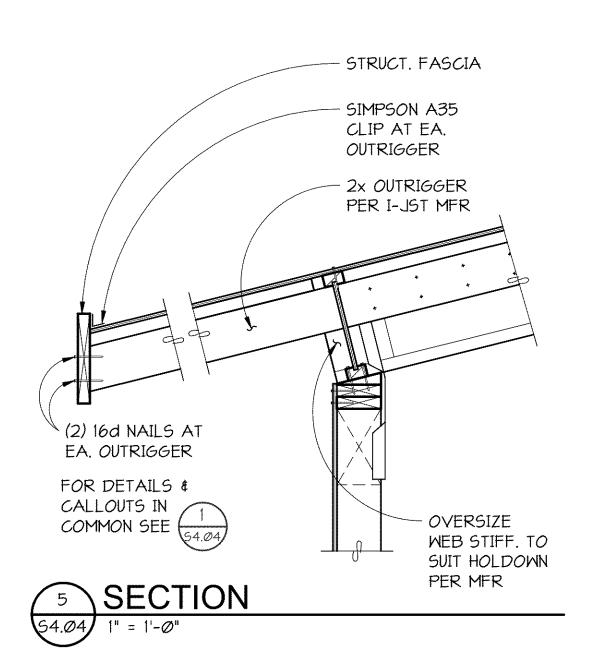


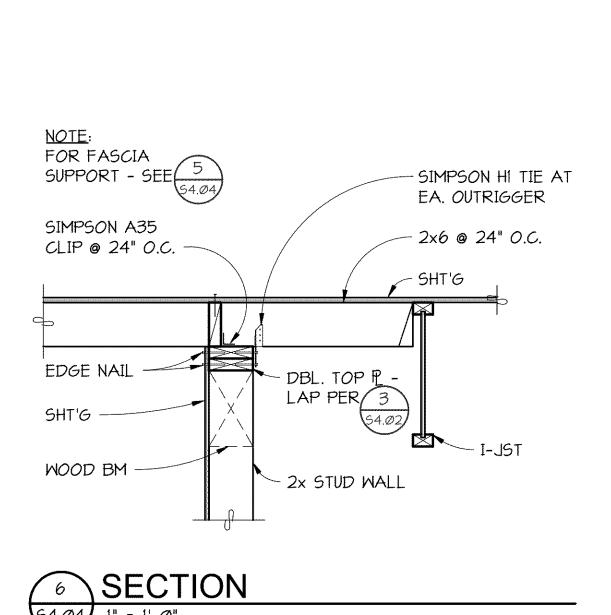


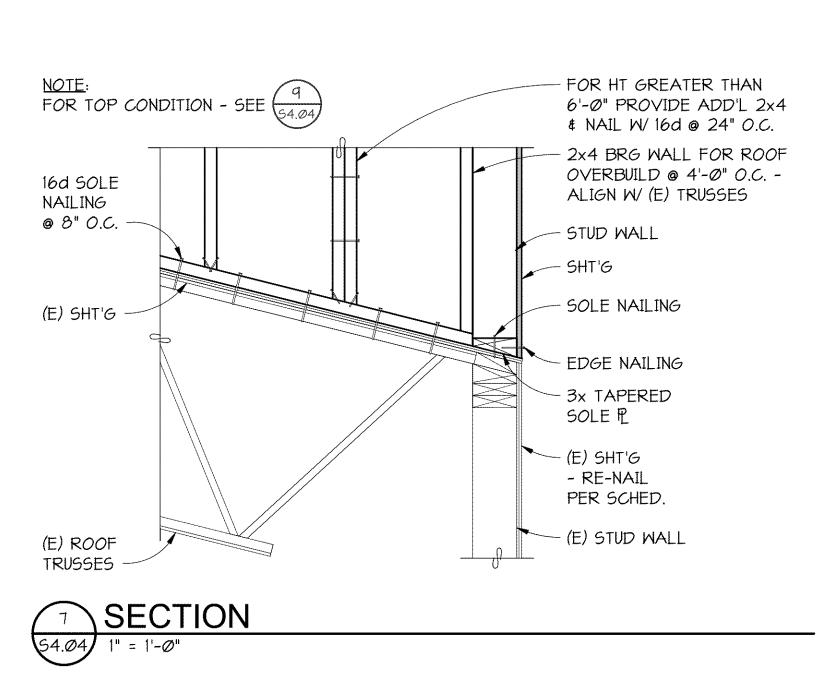


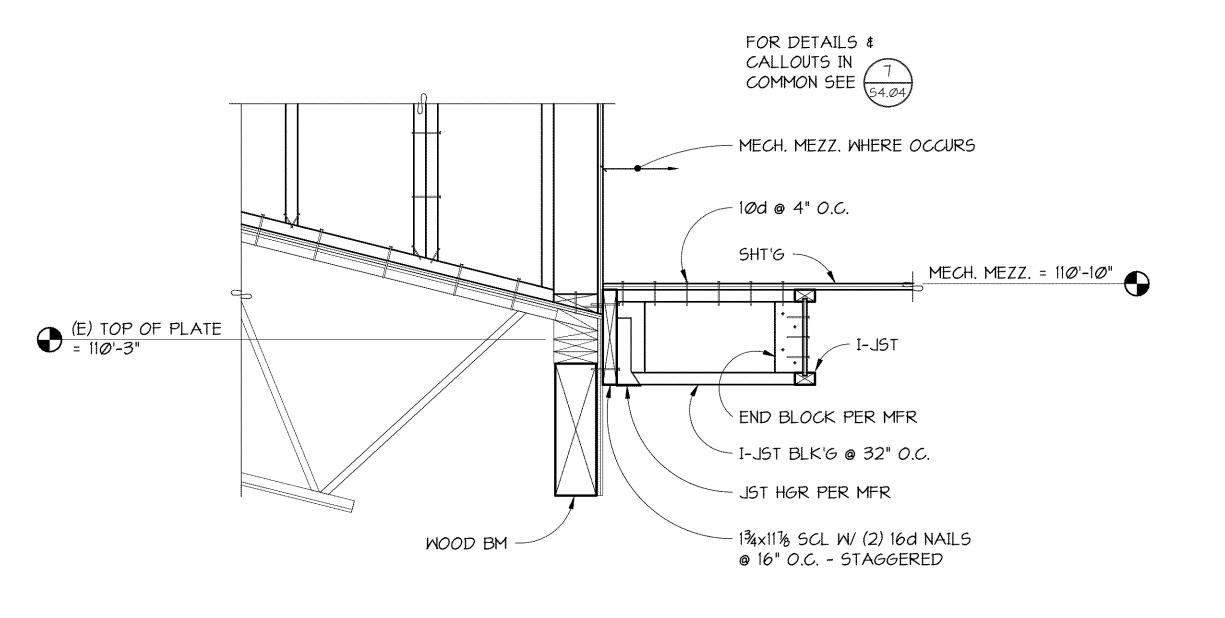




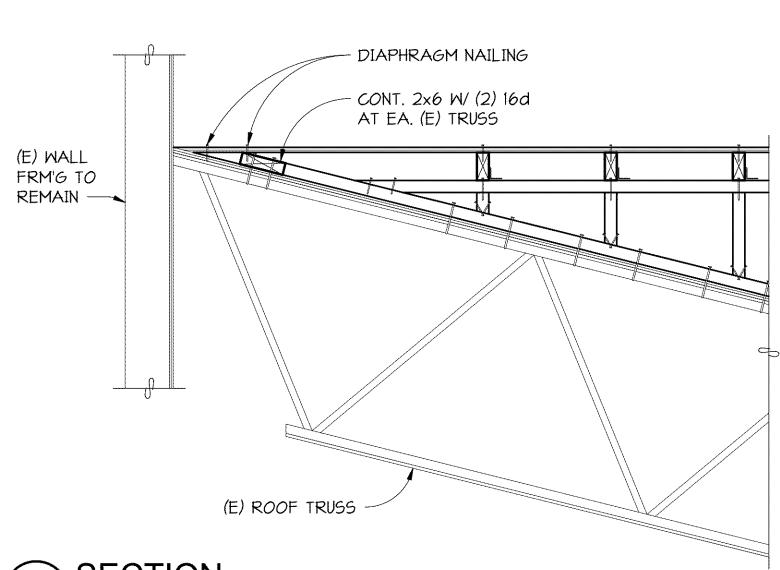


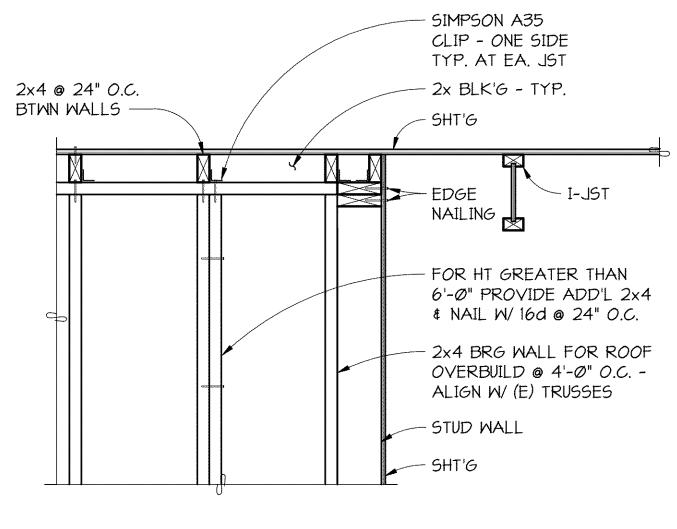












2x4 BRG WALL FO OVERBUILD @ 4'-Ø ALIGN W (E) TRUSS STUD WALL SHT'G **bassetti** | architects

Jurisdiction Stamp Area

71 Columbia Street, Suite 500 Seattle, Washington 98104 T (206) 340 9500 F (206) 340 9519

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STRUCTURAL ENGINEER
PCS Structural Solutions
One Main Place
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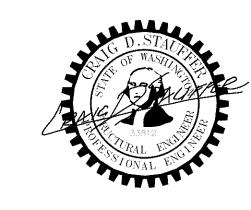
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COST CONSULTANT
Rider Levett Bucknall (RLB)
Brewery Block 2
1120 NW Couch Street, Suite 730
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# REVISIONS DATE

FRANKLIN
ELEMENTARY
SCHOOL
ADDITION

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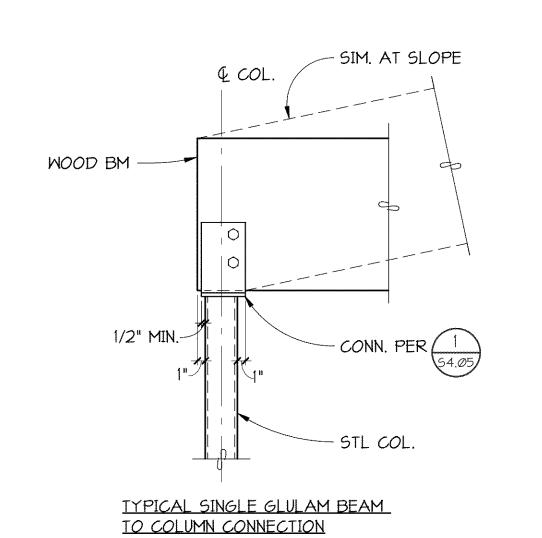
JOB NO: ISSUE DATE:

SSUE DATE: 08/

WOOD FRAMING DETAILS

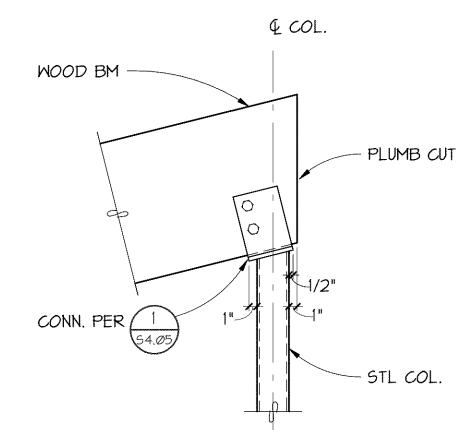
GLULAM BEAM WIDTH	BEARING PLATE "t <sub>i</sub> "	SIDE PLATE "t <sub>2</sub> "	WELD "a"	WELD "b"	MINIMUM MACHINE BOLTS SIZE
31/8	1/2"	3/16"	3/16"		3/4"
5%	5/8"	3/16"	3/16"		3/4"
6 <sup>3</sup> / <sub>4</sub>	3/4"	1/4"	1/4"	1/8"	3/4"

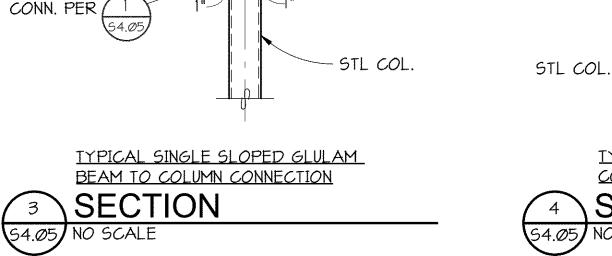
TYPICAL GLULAM BEAM BEARING CONNECTION ASSEMBLY **DETAIL** 54.05 NO SCALE

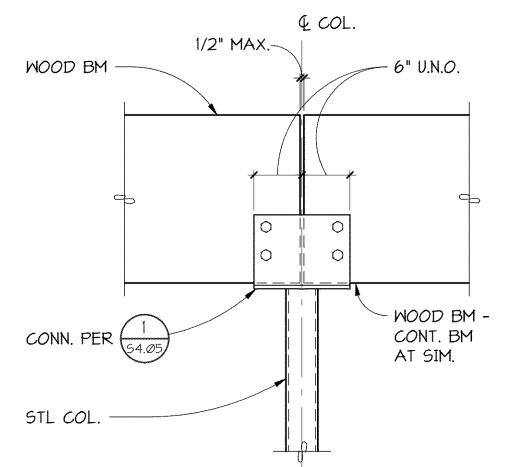


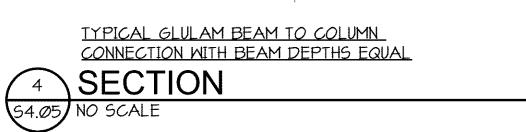
<sup>2</sup> SECTION

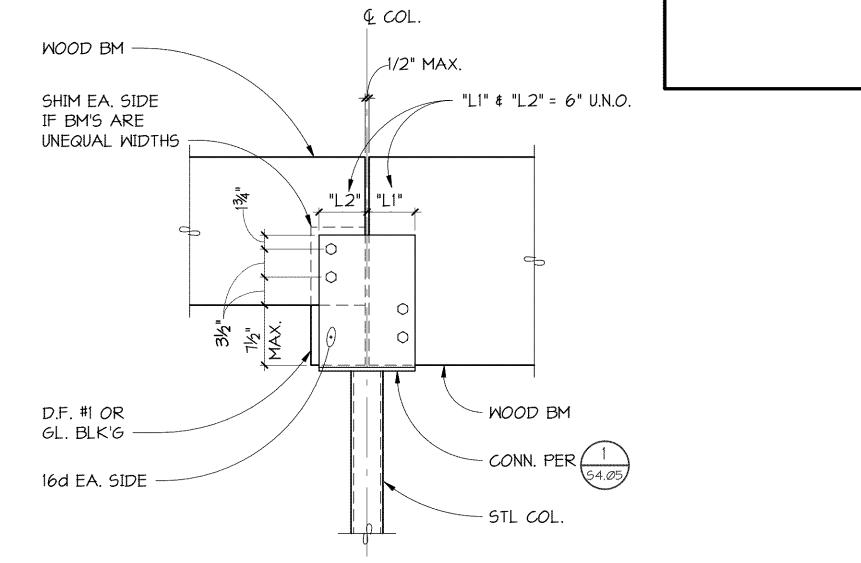
54.05 NO SCALE











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architects

71 Columbia Street, Suite 500 Seattle, Washington 98104

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CIVIL ENGINEER AND LANDSCAPE

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101 SW Main Street, Suite 280

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6021 12th St E, Suite 200

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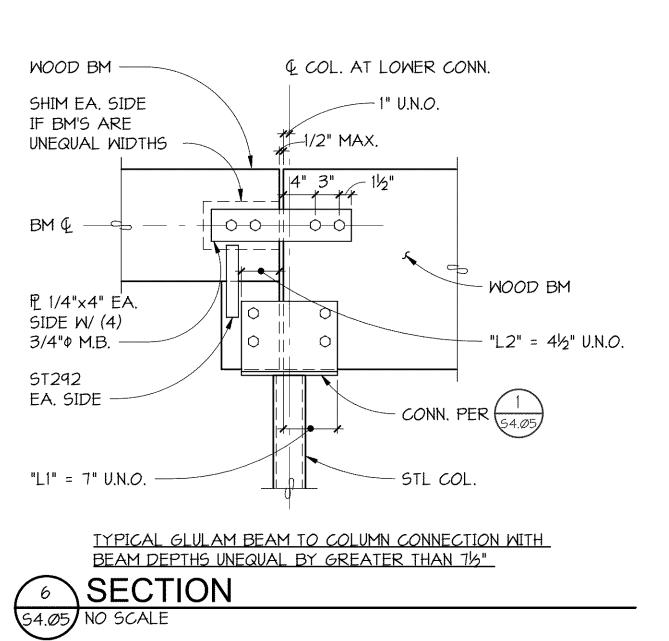
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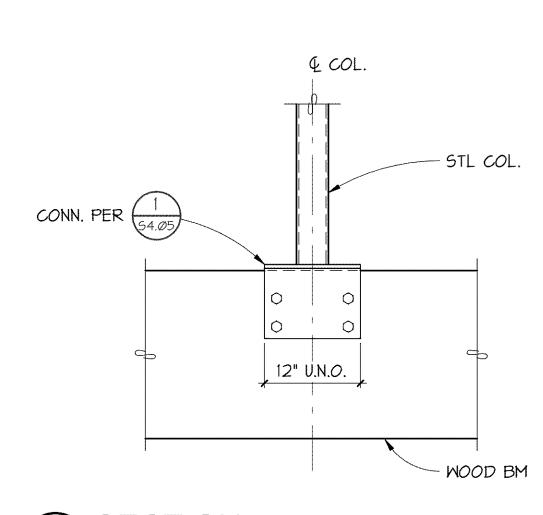
Brewery Block 2

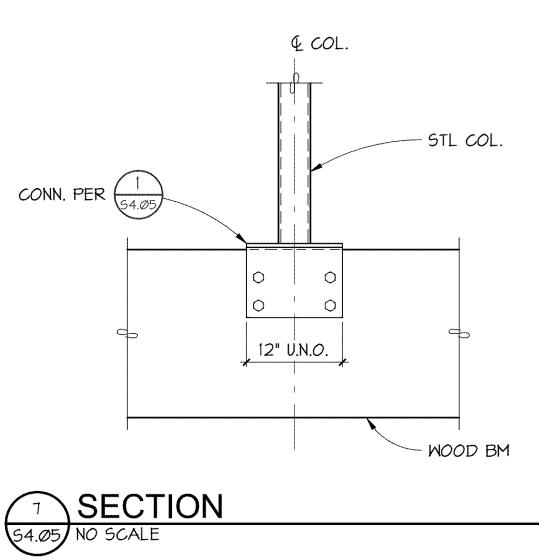
Portland, OR 97209 T (503) 226 2730

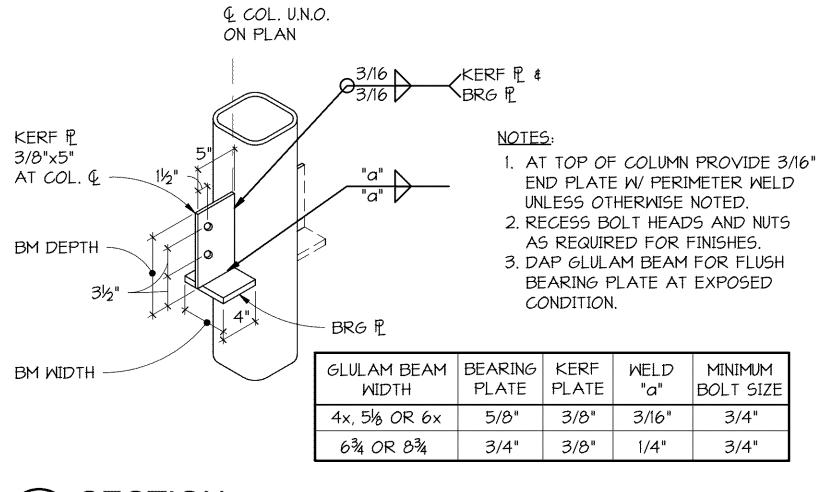
TYPICAL GLULAM BEAM TO COLUMN CONNECTION WITH BEAM DEPTHS UNEQUAL BY 71/3" OR LESS

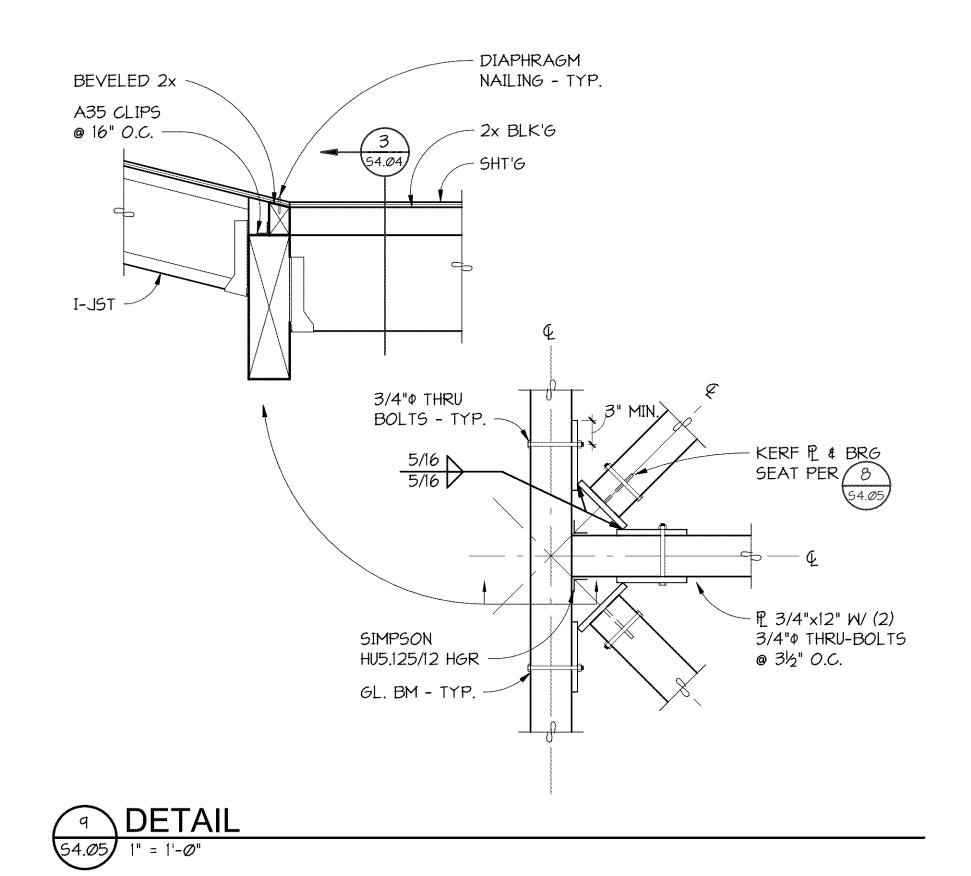




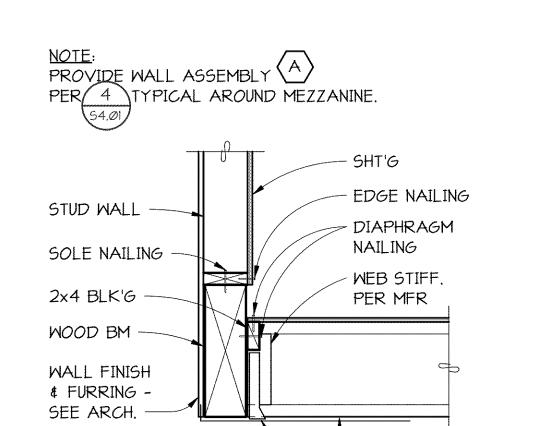








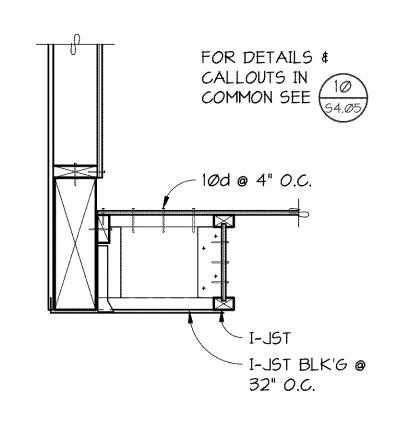
	8	SECTION
i	54.05	NO SCALE

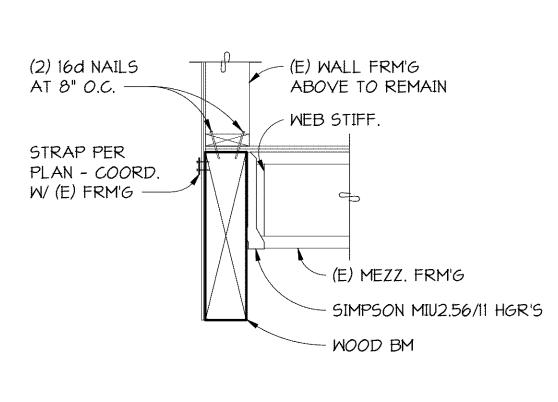


SECTION SECTION

54.05 1" = 1'-0"

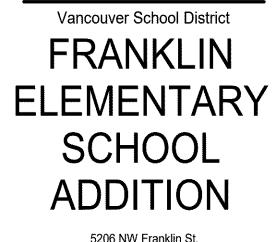
- JST HGR PER MFR











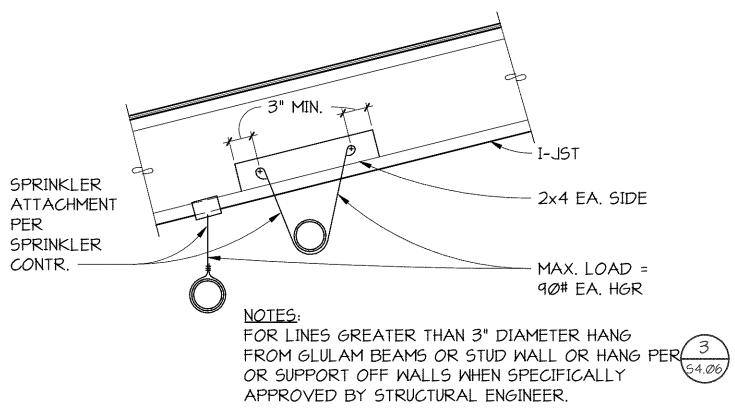
REVISIONS DATE

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ISSUE DATE:

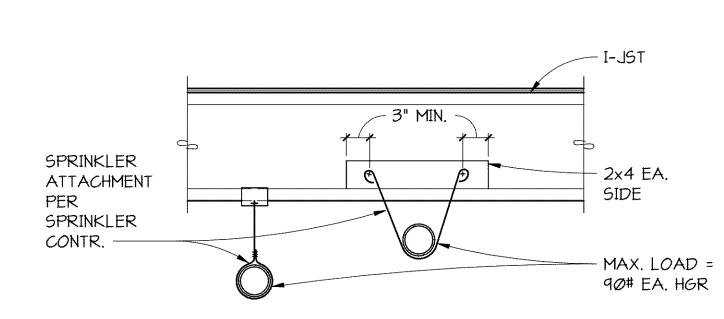
Stamp Area

WOOD FRAMING **DETAILS** 



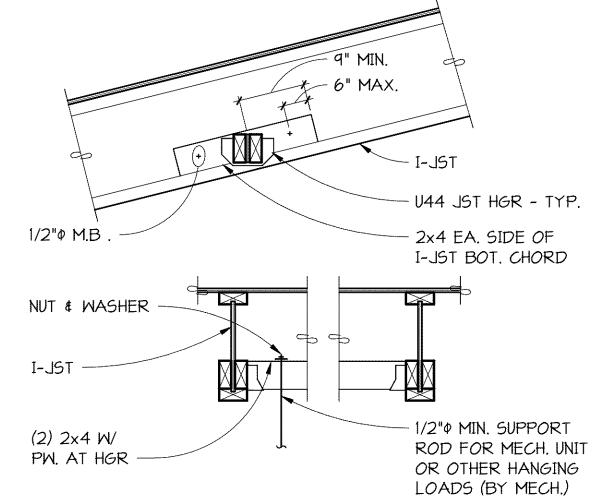
TYPICAL SPRINKLER LINE ATTACHMENT FOR LINES 3" DIAMETER OR SMALLER SECTION

54.06 NO SCALE



FOR LINES GREATER THAN 3" DIAMETER HANG FROM GLULAM BEAMS OR STUD WALL OR HANG PER
OR SUPPORT OFF WALLS WHEN SPECIFICALLY OR SUPPORT OFF WALLS WHEN SPECIFICALLY APPROVED BY STRUCTURAL ENGINEER.

TYPICAL SPRINKLER LINE ATTACHMENT FOR LINES 3" DIAMETER OR SMALLER 2 SECTION

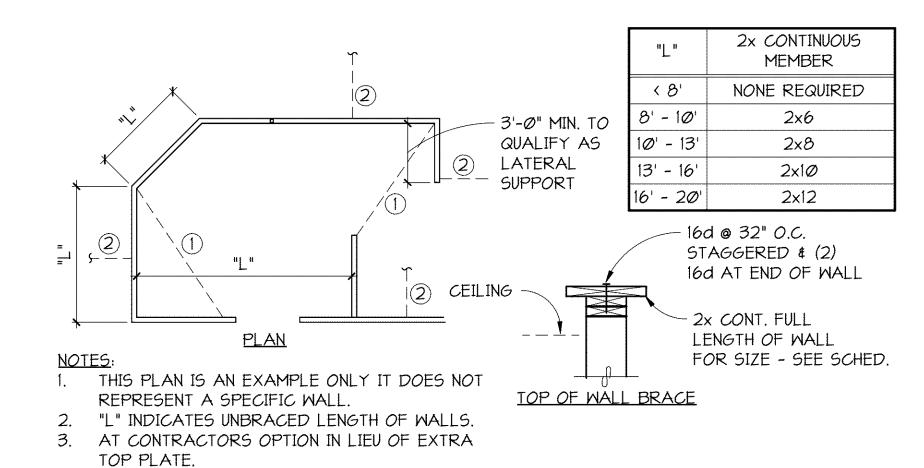


1. ALL FRAMING SHOWN BY GENERAL CONTRACTOR (EXCEPT BY SPRINKLER CONTRACTOR IF USED FOR SPRINKLER LINES.)

DO NOT CUT OR DRILL THRU JOIST MAXIMUM SUPPORT LOAD 500 POUNDS.

> TYPICAL DETAIL FOR HANGING LOADS FROM I-JOIST (ALL HEAT PUMPS OR FANS OVER 90 POUNDS)

SECTION 54.06 NO SCALE



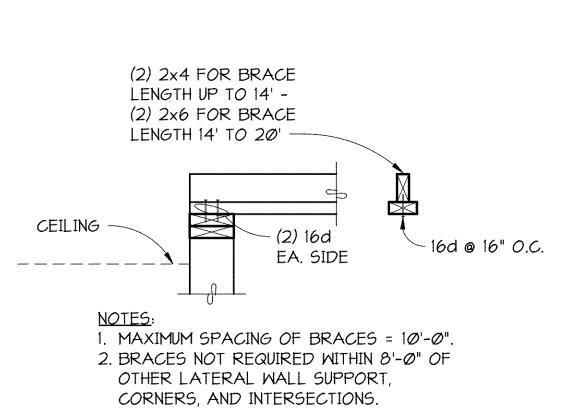
(1) INDICATES HORIZONTAL BRACE EXTENDING TO ADJACENT CORNER - SEE 5/S4.06.

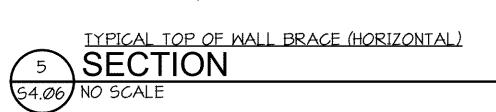
(2) INDICATES BRACE UP TO ROOF STRUCTURE - SEE 6/S4.06, 7/S4.06, 8/S4.06, AND 9/S4.06.

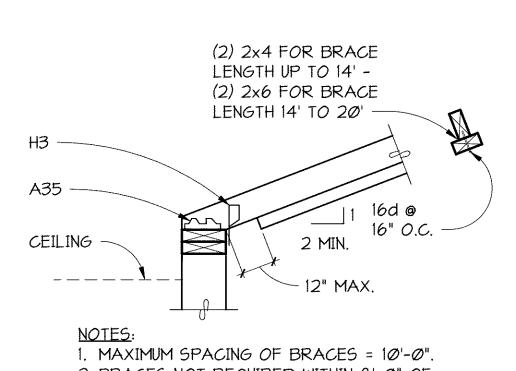
TYPICAL LATERAL SUPPORT FOR INTERIOR NON-BEARING WALLS NOT

EXTENDING TO STRUCTURE



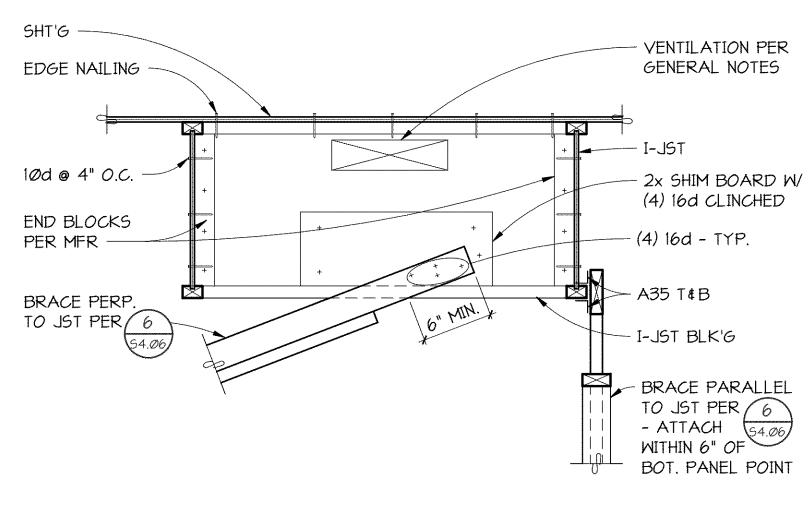


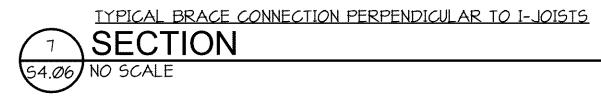


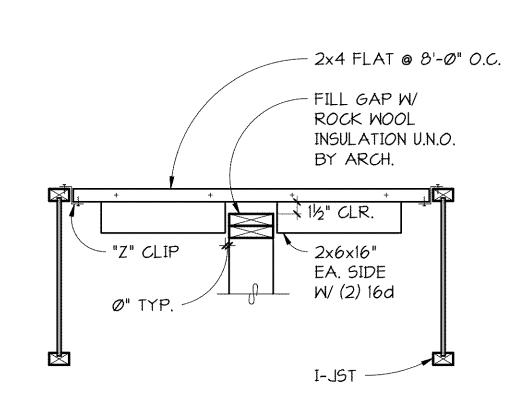


2. BRACES NOT REQUIRED WITHIN 8'-0" OF OTHER LATERAL WALL SUPPORT, CORNERS, AND INTERSECTIONS.

TYPICAL TOP OF WALL BRACE UP TO STRUCTURE **SECTION** 54.06 NO SCALE

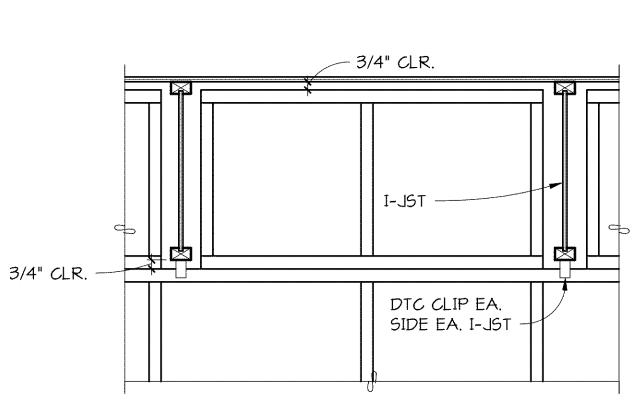






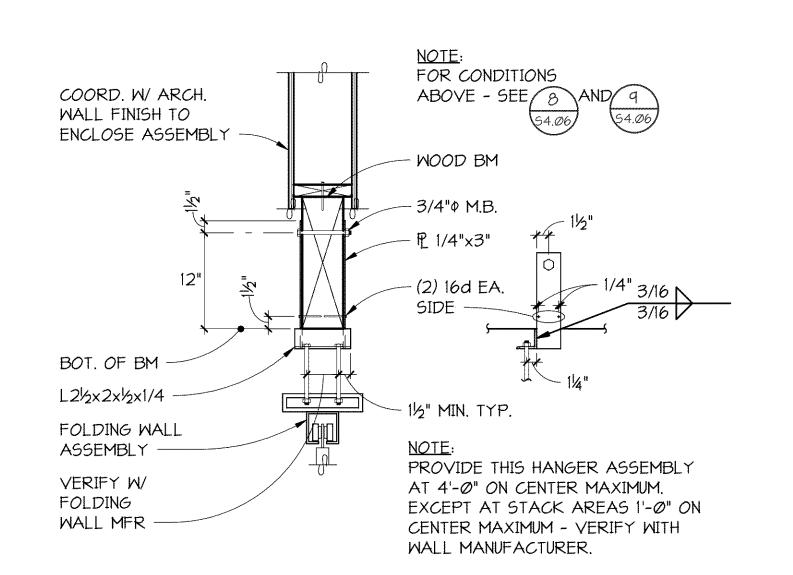
TYPICAL CONNECTION AT TOP OF NON-BEARING WALL EXTENDING TO ROOF STRUCTURE PARALLEL TO JOIST



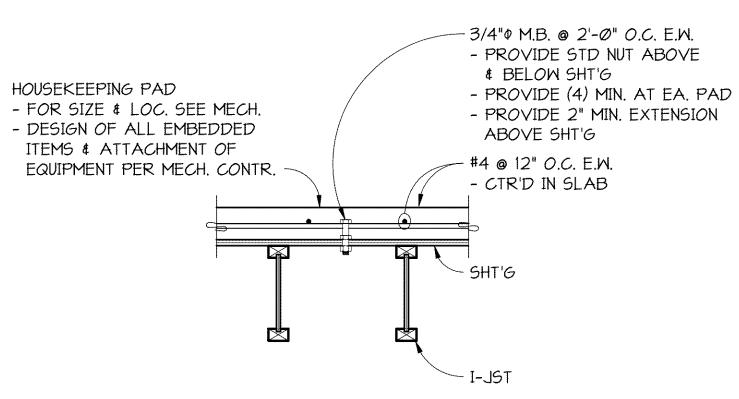


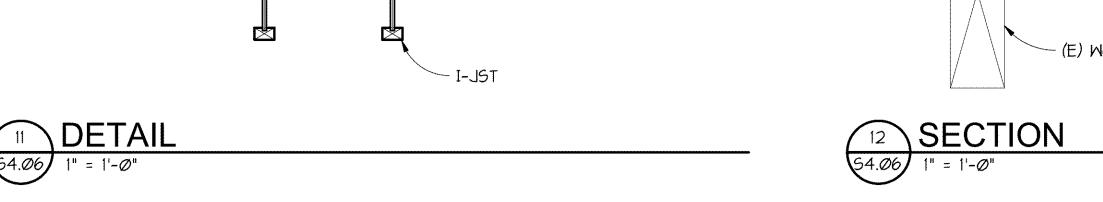
TYPICAL CONNECTION AT TOP OF NON-BEARING WALL EXTENDING TO ROOF STRUCTURE PERPENDICULAR TO JOISTS

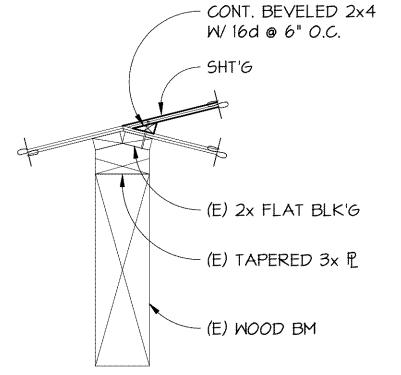
SECTION 54.06 NO SCALE



TYPICAL FOLDING WALL SUPPORT AT WOOD BEAM SECTION SECTION







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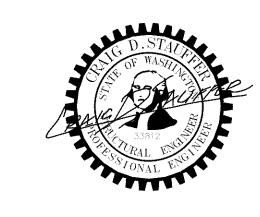
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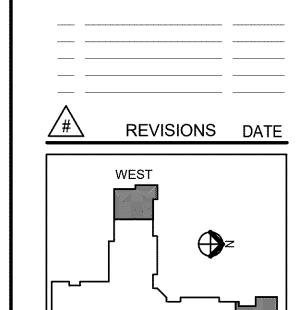
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WOOD **FRAMING DETAILS**